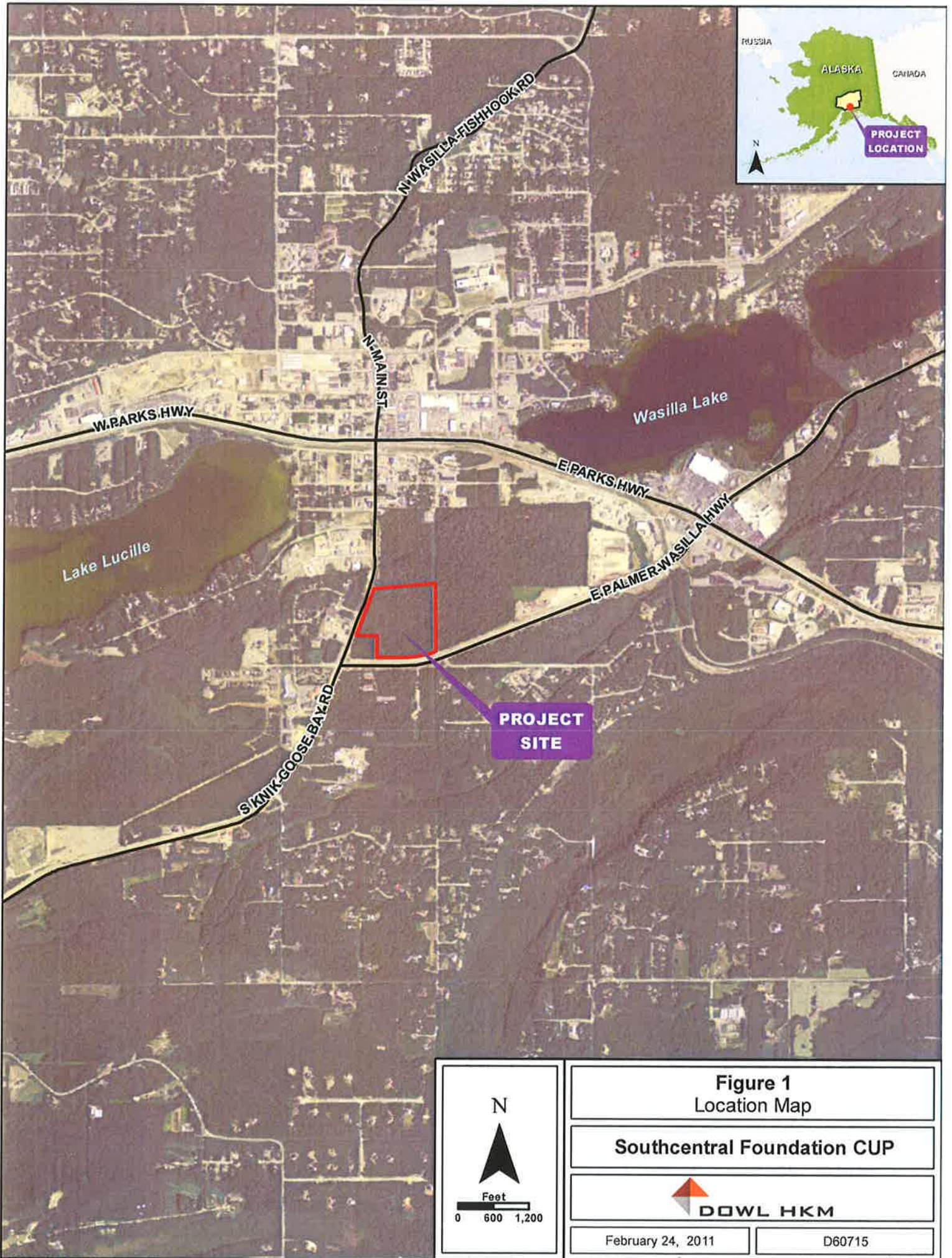


FIGURES

Figure 1 - Location Map



PROJECT SITE

N

Feet

0 600 1,200

Figure 1
Location Map

Southcentral Foundation CUP

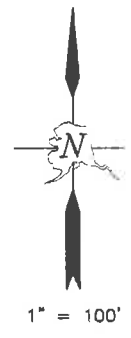
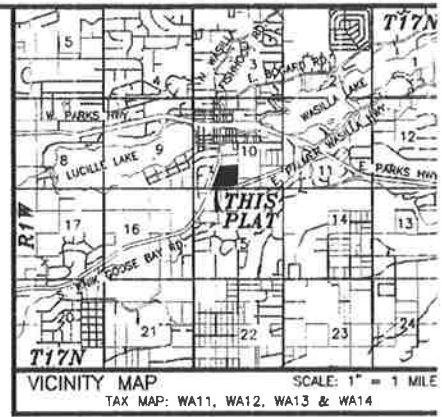
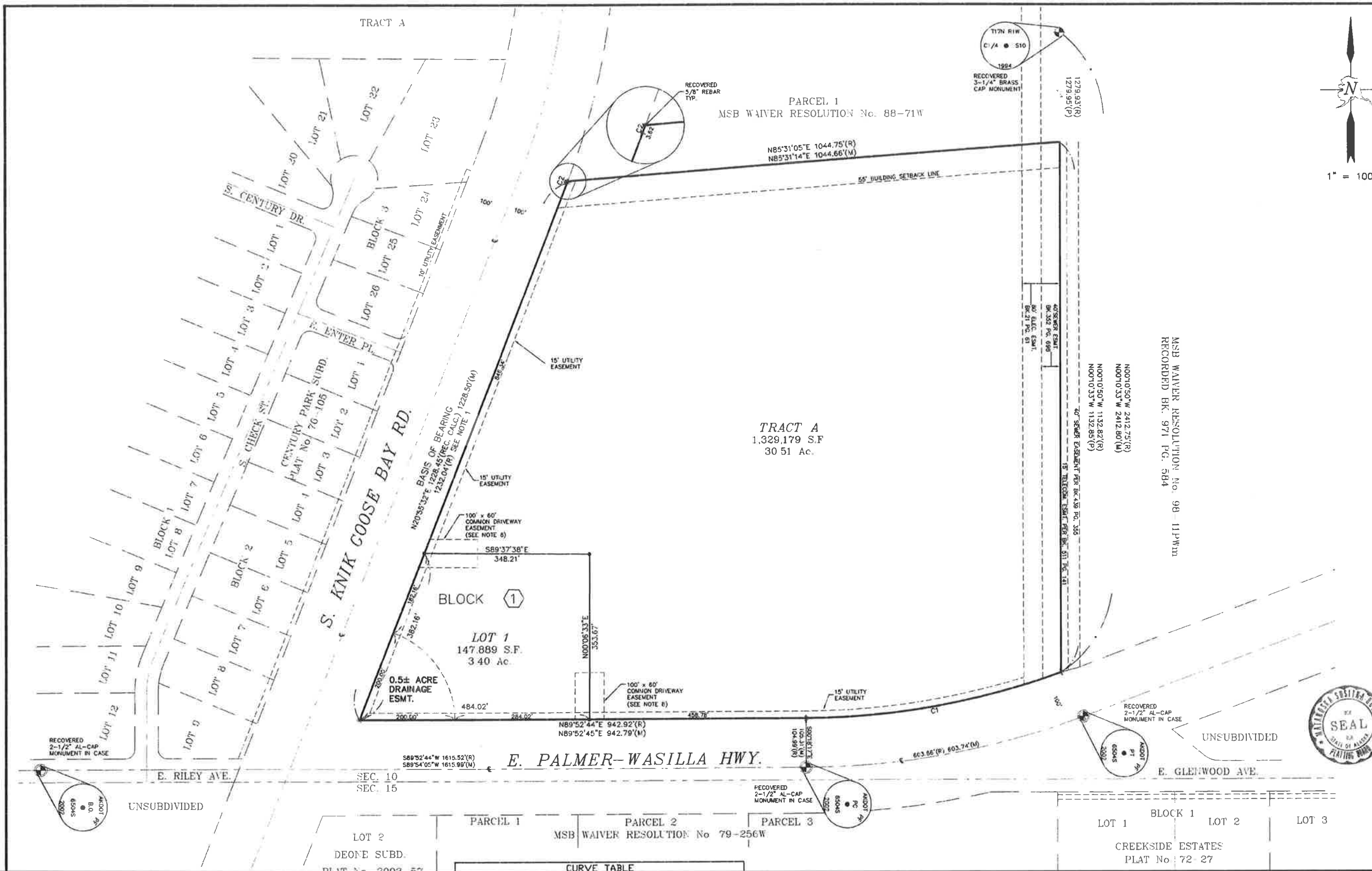


February 24, 2011

D60715

APPENDIX A

Plat



TRACT A
 PARCEL 1
 MSB WAIVER RESOLUTION No. 88-71W

TRACT A
 1,329,179 S.F.
 30.51 Ac.

BLOCK 1

LOT 1
 147,889 S.F.
 3.40 Ac.

E. PALMER-WASILLA HWY.

MSB WAIVER RESOLUTION No. 98-11PWW
 RECORDED BK. 971 PG. 58A

CERTIFICATE OF OWNERSHIP

I HEREBY CERTIFY THAT I AM THE OWNER OF THE PROPERTY SHOWN AND DESCRIBED HEREON AND THAT I HEREBY ADOPT THIS PLAT OF SUBDIVISION BY MY FREE CONSENT AND GRANT ALL EASEMENTS TO THE USE SHOWN.

Gerald E. Neeser 12/1/06
 GERALD E. NEESER, PRESIDENT DATE
 NEESER CONSTRUCTION, INC.
 2501 BLUEBERRY
 ANCHORAGE, AK 99503

NOTARY ACKNOWLEDGEMENT

SUBSCRIBED AND SWORN TO BEFORE ME THIS 1st DAY OF December, 2006 FOR Gerald E. Neeser

Patricia S. ...
 NOTARY FOR THE STATE OF ALASKA
 MY COMMISSION EXPIRES 11/23/09

CERTIFICATE OF PAYMENT OF TAXES

I HEREBY CERTIFY THAT ALL CURRENT TAXES AND SPECIAL ASSESSMENTS, THROUGH December 5, 2006, AGAINST THE PROPERTY, INCLUDED IN THIS SUBDIVISION OR RESUBDIVISION HEREON HAVE BEEN PAID.

... 12/5/06
 CITY OF WASILLA TAX COLLECTION OFFICIAL DATE

CERTIFICATE OF PAYMENT OF TAXES

I HEREBY CERTIFY THAT ALL CURRENT TAXES AND SPECIAL ASSESSMENTS, THROUGH 12-31-2006, AGAINST THE PROPERTY, INCLUDED IN THIS SUBDIVISION OR RESUBDIVISION HEREON HAVE BEEN PAID.

... 12-7-06
 BOROUGH TAX COLLECTION OFFICIAL DATE

PLANNING & LAND USE DIRECTOR'S CERTIFICATE

I CERTIFY THAT THIS SUBDIVISION PLAT HAS BEEN FOUND TO COMPLY WITH THE LAND SUBDIVISION REGULATIONS OF THE MATANUSKA-SUSITNA BOROUGH, AND THAT THE PLAT HAS BEEN APPROVED BY THE PLATTING AUTHORITY BY PLAT RESOLUTION No. 2006-246-50 DATED AUG 3, 2006, AND THAT THIS PLAT HAS BEEN APPROVED FOR RECORDING IN THE OFFICE OF THE RECORDER IN THE PALMER RECORDING DISTRICT, THIRD JUDICIAL DISTRICT, STATE OF ALASKA.

M. ... 12-7-06
 PLANNING AND LAND USE DIRECTOR DATE
...
 ATTEST: PLATTING CLERK



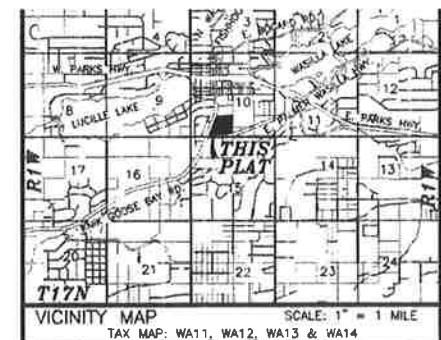
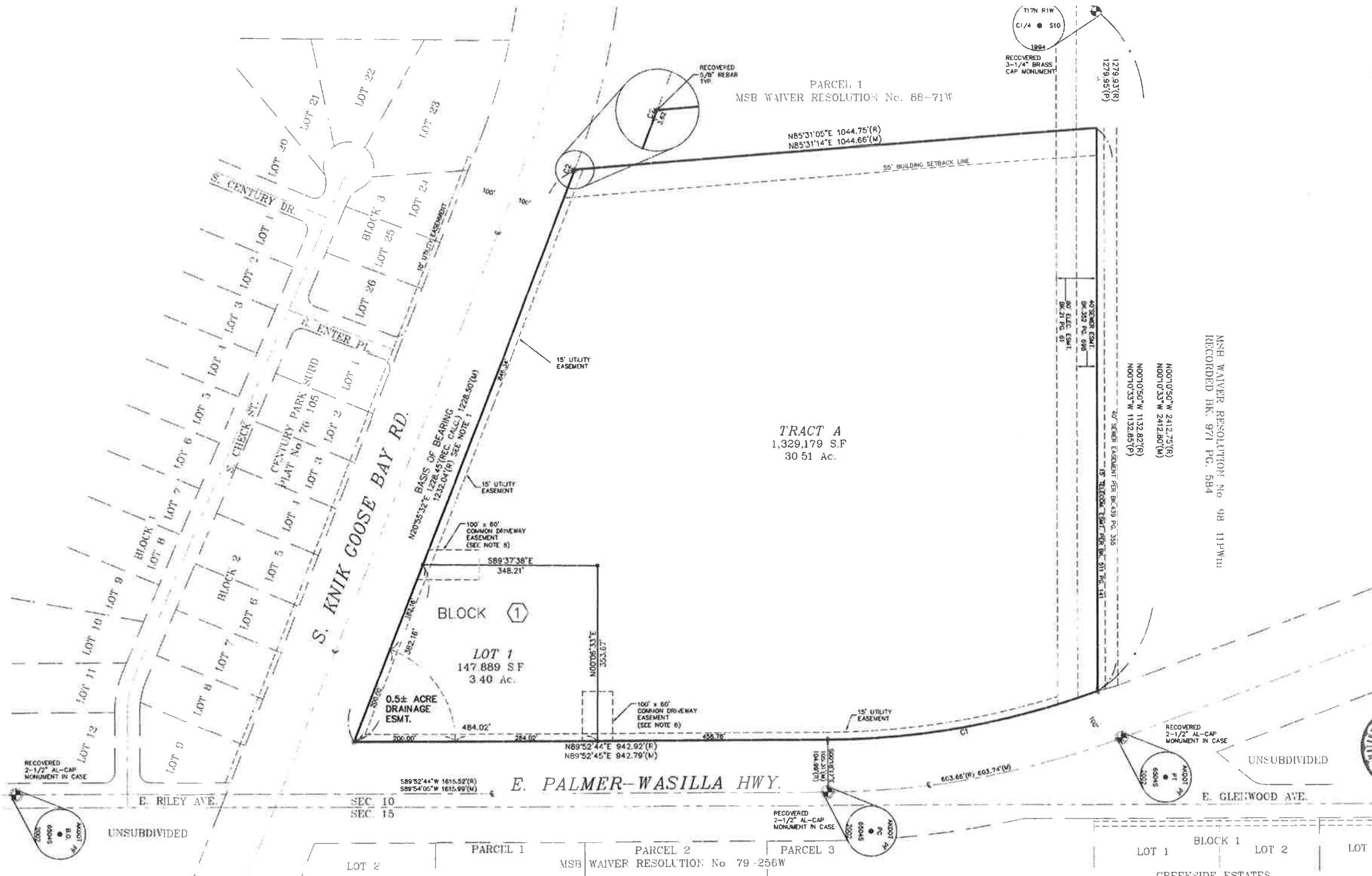
2006-204
 PALMER REC DIST
 Date 12-7-2006
 Time 3:03 PM
 Requested by MSB
 Address

LOT 1	BLOCK 1	LOT 2	LOT 3
CREEKSIDE ESTATES			
PLAT No. 72-27			

PARCEL 1
 MSB WAIVER RESOLUTION No. 79-256W

LOT 2
 DEONE SUBD.
 PLAT No. 2002-57

CURVE TABLE



CERTIFICATE OF OWNERSHIP
 I HEREBY CERTIFY THAT I AM THE OWNER OF THE PROPERTY SHOWN AND DESCRIBED HEREON AND THAT I HEREBY ADOPT THIS PLAT OF SUBDIVISION BY MY FREE CONSENT AND GRANT ALL EASEMENTS TO THE USE SHOWN.
 GERALD E. NEESER, PRESIDENT
 NEESER CONSTRUCTION, INC.
 2501 BLUEBERRY
 ANCHORAGE, AK 99503
 DATE: 12/11/06

NOTARY ACKNOWLEDGEMENT
 SUBSCRIBED AND SWORN TO BEFORE ME THIS 11th DAY OF December, 2006 FOR Gerald E. Neeser
 NOTARY FOR THE STATE OF ALASKA
 MY COMMISSION EXPIRES 11/23/07

CERTIFICATE OF PAYMENT OF TAXES
 I HEREBY CERTIFY THAT ALL CURRENT TAXES AND SPECIAL ASSESSMENTS, THROUGH December 31, 2006, AGAINST THE PROPERTY, INCLUDED IN THIS SUBDIVISION OR RESUBDIVISION, HEREON HAVE BEEN PAID.
 CITY OF WASILLA TAX COLLECTION OFFICIAL DATE: 12/15/06

CERTIFICATE OF PAYMENT OF TAXES
 I HEREBY CERTIFY THAT ALL CURRENT TAXES AND SPECIAL ASSESSMENTS, THROUGH Dec 2006, AGAINST THE PROPERTY, INCLUDED IN THIS SUBDIVISION OR RESUBDIVISION, HEREON HAVE BEEN PAID.
 BOROUGH TAX COLLECTION OFFICIAL DATE: 12-7-06

PLANNING & LAND USE DIRECTOR'S CERTIFICATE
 I CERTIFY THAT THIS SUBDIVISION PLAT HAS BEEN FOUND TO COMPLY WITH THE LAND SUBDIVISION REGULATIONS OF THE MATANUSKA-SUSITNA BOROUGH, AND THAT THE PLAT HAS BEEN APPROVED BY THE PLATTING AUTHORITY BY PLAT RESOLUTION No. 2006-246-246 DATED 12/6/06, 2006, AND THAT THIS PLAT HAS BEEN APPROVED FOR RECORDING IN THE OFFICE OF THE RECORDER IN THE PALMER RECORDING DISTRICT, THIRD JUDICIAL DISTRICT, STATE OF ALASKA.
 PLANNING AND LAND USE DIRECTOR DATE: 12-7-06
 ATTEST: PLATTING CLERK



CURVE TABLE

CURVE	LENGTH	RADIUS	DELTA	TANGENT	CHORD BRNG.	CHORD LENGTH
C1(R)	553.82	1568.21	202.403°	279.83	N09°47'40"E	550.95
C1(M)	553.78	1568.21	201.738°	279.80	N09°47'36"E	550.91
C2(R)	3.59	1532.50	000°08'01"	1.79	N60°31'30"E	3.59
C2(M)	3.62	1532.50	000°08'07"	1.81	N60°28'10"E	3.62

- LEGEND**
- * RECOVERED 5/8" REBAR
 - RECOVERED 3-1/4" BRASS CAP MONUMENT
 - RECOVERED 2-1/2" ALUMINUM CAP MONUMENT IN DOT CASE
 - (R) RECORD DATA FROM RECORD OF SURVEY No. 2003-100
 - (P) PRORATED DATA THIS PLAT
 - (C) CALCULATED DATA THIS PLAT
 - (M) MEASURED DATA THIS PLAT
 - SET 5/8"x24" REBAR w/PLASTIC CAP MARKED AX RIM 2234-S

SURVEYOR'S CERTIFICATE
 I HEREBY CERTIFY THAT I AM A REGISTERED PROFESSIONAL LAND SURVEYOR IN THE STATE OF ALASKA AND THAT THIS PLAT REPRESENTS A SURVEY MADE BY ME OR UNDER MY DIRECT SUPERVISION, AND THAT THE MONUMENTS SHOWN THEREON ACTUALLY EXIST AS DESCRIBED AND THAT ALL DIMENSIONAL AND OTHER DETAILS ARE TRUE AND CORRECT TO THE BEST OF MY KNOWLEDGE.
 DATE: Dec. 1, 2006



- NOTES**
- RECORD OF SURVEY No. 2003-100 DOES NOT CLOSE MATHEMATICALLY. DISTANCE 1232.04' APPEARING ON THE WESTERLY LINE APPEARS TO INCLUDE THE 3.59' CURVE AT THE NORTH END.
 - THERE MAY BE FEDERAL, STATE AND LOCAL REQUIREMENTS GOVERNING LAND USE. IT IS THE RESPONSIBILITY OF THE INDIVIDUAL PARCEL OWNER TO OBTAIN A DETERMINATION WHETHER SUCH REQUIREMENTS APPLY TO THE DEVELOPMENT OF THE PARCELS SHOWN HEREON.
 - NO INDIVIDUAL WATER SUPPLY SYSTEM OR SEWAGE DISPOSAL SYSTEM SHALL BE PERMITTED ON ANY LOT.
 - ALL LOTS ARE SERVED BY CITY OF WASILLA SEWER AND WATER SYSTEMS.
 - RECORD INFORMATION WAS TAKEN FROM RECORD OF SURVEY RECORDED No. 2003-100
 - DIRECT ACCESS FOR LOT 1, BLOCK 1 ONTO E. PALMER-WASILLA HWY. AND S. KNIK-GOOSE BAY RD. IS RESTRICTED TO THE COMMON DRIVEWAY ACCESS EASEMENTS AS SHOWN.
 - THERE IS LESS THAN ONE PERCENT CHANCE THAT ANY PART OF THE PLATTED AREA WILL BE INUNDATED BY THE BASE FLOOD EVENT IN ANY GIVEN YEAR.
 - 100' x 60' COMMON DRIVEWAY EASEMENT FOR INGRESS/EGRESS ACCESS BETWEEN LOT 1, BLOCK 1 AND TRACT A.

2006-204
 PLAT No. REC. DIST.
 Date: 12-7-2006
 Time: 3:03 PM
 Requested By: MSB
 Address:

A PLAT OF
ROCK CENTER PHASE 1
 A SUBDIVISION OF
PARCEL No. 2, MSB WAIVER RESOLUTION RECORDING No. 88-71W (EXCEPTING THEREFROM ANY PORTION OF THE E. PALMER WASILLA HIGHWAY RIGHTS OF WAY) PALMER RECORDING DISTRICT
 LOCATED WITHIN THE SW1/4, SECTION 10, T17N, R1W, S.M., AK. CONTAINING 33.9± ACRES
ALASKA RIM ENGINEERING, INC.
 ENGINEERS-PLANNERS-SURVEYORS
 P.O. BOX 8749 PALMER, ALASKA 99645 (907) 746-0885 FAX (907) 746-0821
 W.D. 0600191 DATE: DECEMBER, 2006 SCALE: 1" = 100'
 DRAWN BY: JRG FILE: 0600191_PL SHEET 1 OF 1

Palmer 2006-204

AFFIDAVIT

I hereby certify that I hold the herein specified property interest in the property shown and described hereon and that I hereby adopt this plan of subdivision by my free consent and grant all easements to the use shown. {delete inapplicable phrases}

Deed of Trust

Rock Center

Recording Serial No. 2005-020855-0

Phase I

Current Legal Description

Proposed Subdivision

First National Bank Alaska

William P. Insko SVP

(Signature)

Lien Holder

Interest in Property

William P. Insko Senior Vice Pres
(Printed Name and Title)

PO Box 100720
Anchorage, AK 99510
Address

NOTARY CERTIFICATION

State of Alaska)
Third Judicial)SS
County of District)

SUBSCRIBED and SWORN to (or affirmed) before me this 31ST day of August
(month)

2006, by William P Insko
(year) (name of signers)



Beth A. Hayward-Berry
(signature and seal of notary)

My Commission expires: 5-30-2010

Palmer 2006-204

AFFIDAVIT

I hereby certify that I hold the herein specified property interest in the property shown and described hereon and that I hereby adopt this plan of subdivision by my free consent and grant all easements to the use shown. {delete inapplicable phrases}

Deed of Trust	Rock Center
Recording Serial No. 2005-010828-0	Phase I
<p>PORTION OF SW 1/4 SEC 10 T17N R1W <small>Current Legal Description</small></p> <p><u>L. Dickinson</u> ROCK PARTNERS <small>(Signature)</small></p> <p>LEWIS DICKINSON - GENERAL PARTNER <small>(Printed Name and Title)</small></p> <p>1040 O'MALLEY ROAD ANCHORAGE <small>Address</small></p>	<hr/> <p>Proposed Subdivision</p> <p><u>FIRST D.O.T.</u> Interest in Property</p>

NOTARY CERTIFICATION

State of _____)
 County of _____) SS
)

SUBSCRIBED and SWORN to (or affirmed) before me this 6 day of September (month)
 2006, by LEWIS DICKINSON
 (year) (name of signers)



[Signature]
 (signature and seal of notary)
 My Commission expires: 5/28/2010

APPENDIX B

Draft TIA

TRAFFIC IMPACT ANALYSIS

Southcentral Foundation
Valley Medical Complex Phase I
Wasilla, Alaska

February 2011



rendering by: nbbj

TRAFFIC IMPACT ANALYSIS

SOUTHCENTRAL FOUNDATION

VALLEY MEDICAL COMPLEX PHASE I

WASILLA, ALASKA

Prepared for:

Southcentral Foundation
4501 Diplomacy Drive, Suite 200
Anchorage, Alaska 99508

Prepared by:

DOWL HKM
4041 B Street
Anchorage, Alaska 99503
(907) 562-2000

W.O. 60763

February 2011

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APPENDICES

Appendix A	Scoping Meeting Minutes
Appendix B	Traffic Volume Documentation
Appendix C	Background and Total Traffic Analysis
Appendix D	Mitigation Analysis

LIST OF ACRONYMS

AAC	Alaska Administrative Code
AADT	annual average daily traffic
DOT&PF	State of Alaska Department of Transportation and Public Facilities
HCS 2000	Highway Capacity Software 2000
ITE	Institute of Transportation Engineers
LOS	level of service
MASCOT	Mat-Su Community Transit
Mat-Su	Matanuska-Susitna
mph	miles per hour
TIA	Traffic Impact Analysis
vpd	vehicles per day

1.0 INTRODUCTION

1.1 Purpose and Objectives

The purpose of this Traffic Impact Analysis (TIA) is to determine the transportation related impacts of a proposed 100,000-square-foot Valley Medical Building and related parking facilities on Tract A of the Rock Center Subdivision. The property is located at the northeast corner of the intersection of Knik-Goose Bay Road and Palmer-Wasilla Highway in Wasilla, Alaska. Access is proposed via Knik-Goose Bay Road at Enter Way and via Palmer-Wasilla Highway at a midblock location between Knik-Goose Bay Road and Glenwood Avenue. See Figure 1 for a Development and Vicinity Site Map.

The objectives of this TIA are to:

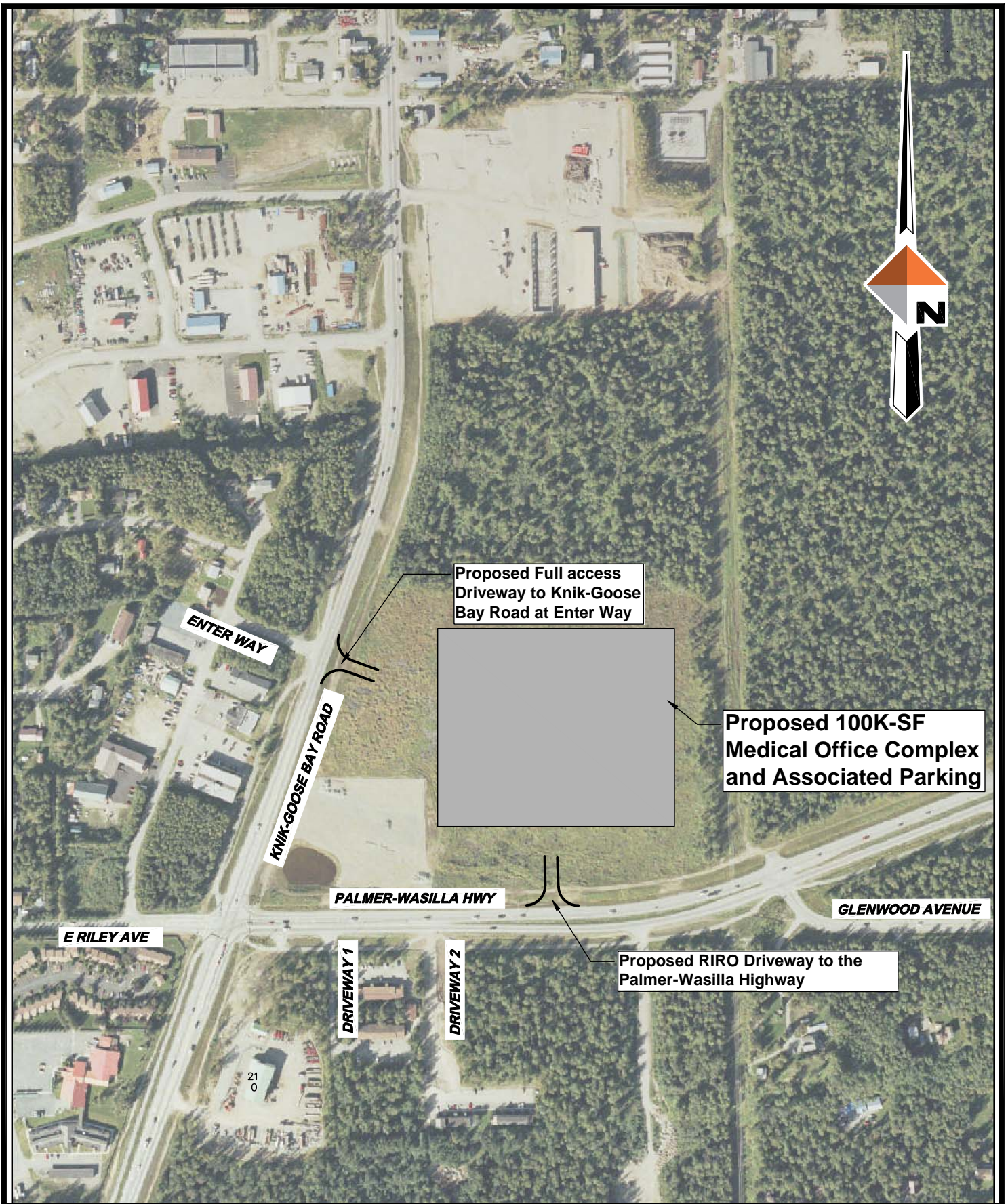
- adequately assess the traffic impacts associated with the proposed development and identify alternatives for off-site access and traffic control mitigation, if required; and
- provide a technically sound basis to identify/negotiate mitigation requirements in response to off-site traffic impacts.

The transportation issues discussed in this TIA include:

- a.m. and p.m. peak-hour estimates for construction and design years traffic conditions without site build-out (referred to as “background”),
- a.m. and p.m. peak-hour estimates for construction and design years total traffic conditions, and
- roadway and access improvements associated with the proposed development necessary to achieve an acceptable level of service (LOS).

1.2 Prior and Current Studies

A traffic impact study was prepared by Tryck Nyman Hayes, Incorporated, for a proposed development on this site in 2006, but was never finalized.



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Vicinity and Development Site Map
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 1

2.0 EXISTING CONDITIONS

2.1 Existing Land Use and Zoning

The site is currently undeveloped. Surrounding land use consists primarily of residential and commercial development accessed via Palmer-Wasilla Highway/Knik-Goose Bay Road intersection, Enter Way, or Glenwood Avenue. The Pioneer Home is located south of the proposed site and is accessed primarily via East Riley Avenue. An apartment complex is located south of Palmer-Wasilla Highway and is accessed directly from Palmer-Wasilla Highway via one full access driveway and one right-in right-out driveway.

The proposed development area is zoned Commercial. The existing zoning for the properties adjacent to the development are Commercial, Rural Residential, and Multi-Family Residential.

2.2 Adjacent Roadways

The project site is accessed via Knik-Goose Bay Road and Palmer-Wasilla Highway. Knik-Goose Bay Road is classified as a Rural Major Collector by the State of Alaska Department of Transportation and Public Facilities (DOT&PF). The roadway section consists of two paved travel lanes and a paved pedestrian pathway on the west side. The segment between Palmer-Wasilla Highway and Parks Highway has a posted speed of 45 miles per hour (mph). It is owned and maintained by the DOT&PF and has a 2009 annual average daily traffic (AADT) of 9,250 vehicles per day (vpd), per DOT&PF Annual Traffic Volume Report 2007-2009.

Palmer-Wasilla Highway is classified as a Rural Minor Arterial by DOT&PF. The roadway section consists of three paved travel lanes, two lanes westbound and one lane eastbound, and a paved pedestrian pathway along the south side. The posted speed is 45 mph, and the right-of-way is owned and maintained by the DOT&PF. This segment of the road has a 2009 AADT of 11,460 vpd, per the DOT&PF Annual Traffic Volume Report 2007-2009.

2.3 Area of Significant Impact

According to Alaska Administrative Code (17 AAC 10.070), a TIA must address:

- intersections on highways where traffic on any approach is expected to increase as a result of the proposed development by at least 5% of the approach's capacity;

- segments of the highways between intersections where total traffic is expected to increase as a result of the proposed development be at least 5% of the segments' capacity;
- state highways and intersections where the safety of the facilities will deteriorate as a result of the traffic generated by the development;
- each driveway or approach road that will allow egress from, or ingress to, a highway for the proposed development;
- parking and circulation routes within the proposed development, to ensure that traffic does not back up onto a highway; and
- pedestrian and bicycle facilities that are part of the highway facilities to which a permit applicant seeks access.

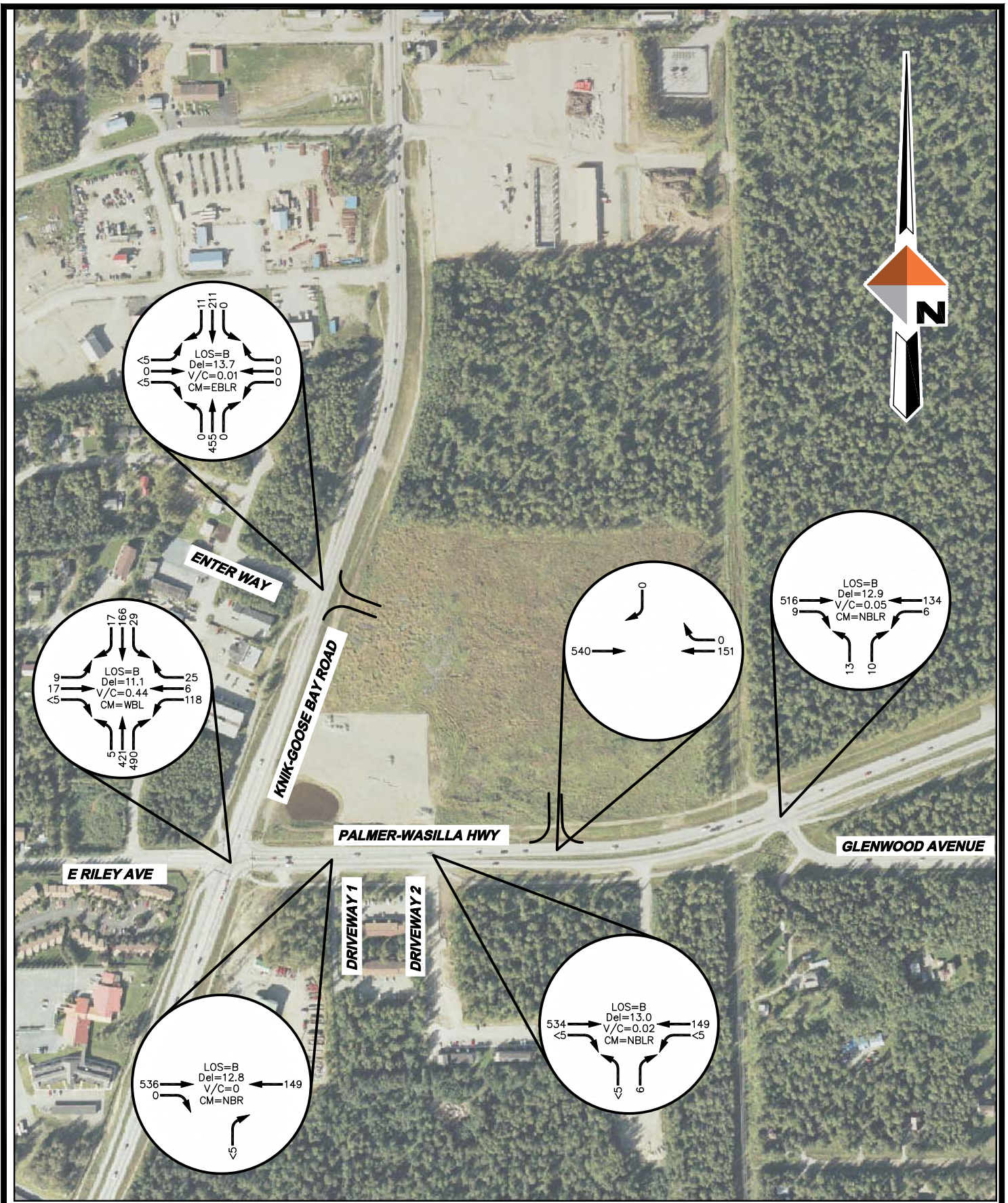
Based on a scoping meeting with the DOT&PF Traffic Engineer, the criteria above, the amount of site-generated traffic, and distribution of trips on the roadway network, this TIA evaluated the following intersections:

- Knik-Goose Bay Road/Enter Way,
- Knik-Goose Bay Road/Palmer-Wasilla Highway,
- Palmer-Wasilla Highway/Glenwood Avenue, and
- Palmer-Wasilla Highway/Site Access.

For complete scoping meeting minutes, refer to Appendix A.

2.4 Traffic Volumes and Conditions

Traffic counts were performed at study intersections on December 1, 2010, during the a.m. and p.m. peak hours. Twenty-four-hour traffic counts were collected via radar on Knik-Goose Bay Road between Palmer-Wasilla Highway and Enter Way. Raw traffic volume documentation is included in Appendix B and depicted in Figures 2 and 3.

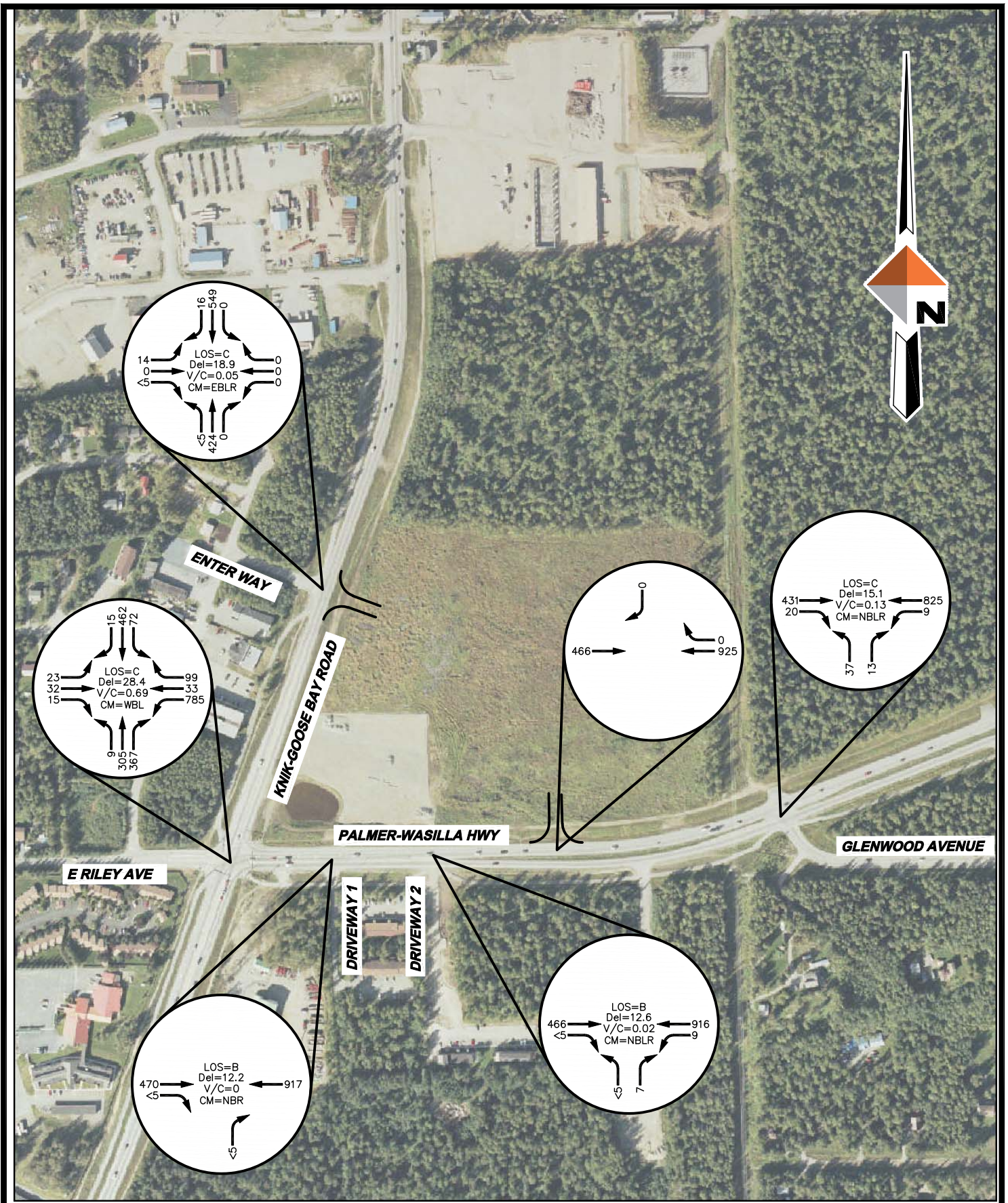


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2010 - A.M. Existing Traffic
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 2



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2010 - P.M. Existing Traffic
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 3

2.5 Anticipated Future Development

The development described in this TIA is Phase I of a potentially four-phase project, however, the development that will be included in future phases is unknown. Traffic impacts of future phases will require reevaluation when the level of development is known.

DOT&PF is planning a future couplet running parallel to Knik-Goose Bay Road. The couplet is planned to connect to Knik-Goose Bay Road at the northwest corner of the property. Future site design should take this into account when determining building footprints and site layout.

DOT&PF has identified a future project to reconstruct Knik-Goose Bay Road with a 4-lane separated cross-section from MP 0.3 to MP 6.8, (between Parks Highway and Vine Road). It is assumed this project will be completed within the next 10 years.

Future signal locations are on Knik-Goose Bay Road at the northwest corner of the site and at the intersection of Glenwood Avenue/Palmer-Wasilla Highway based on correspondence with DOT&PF

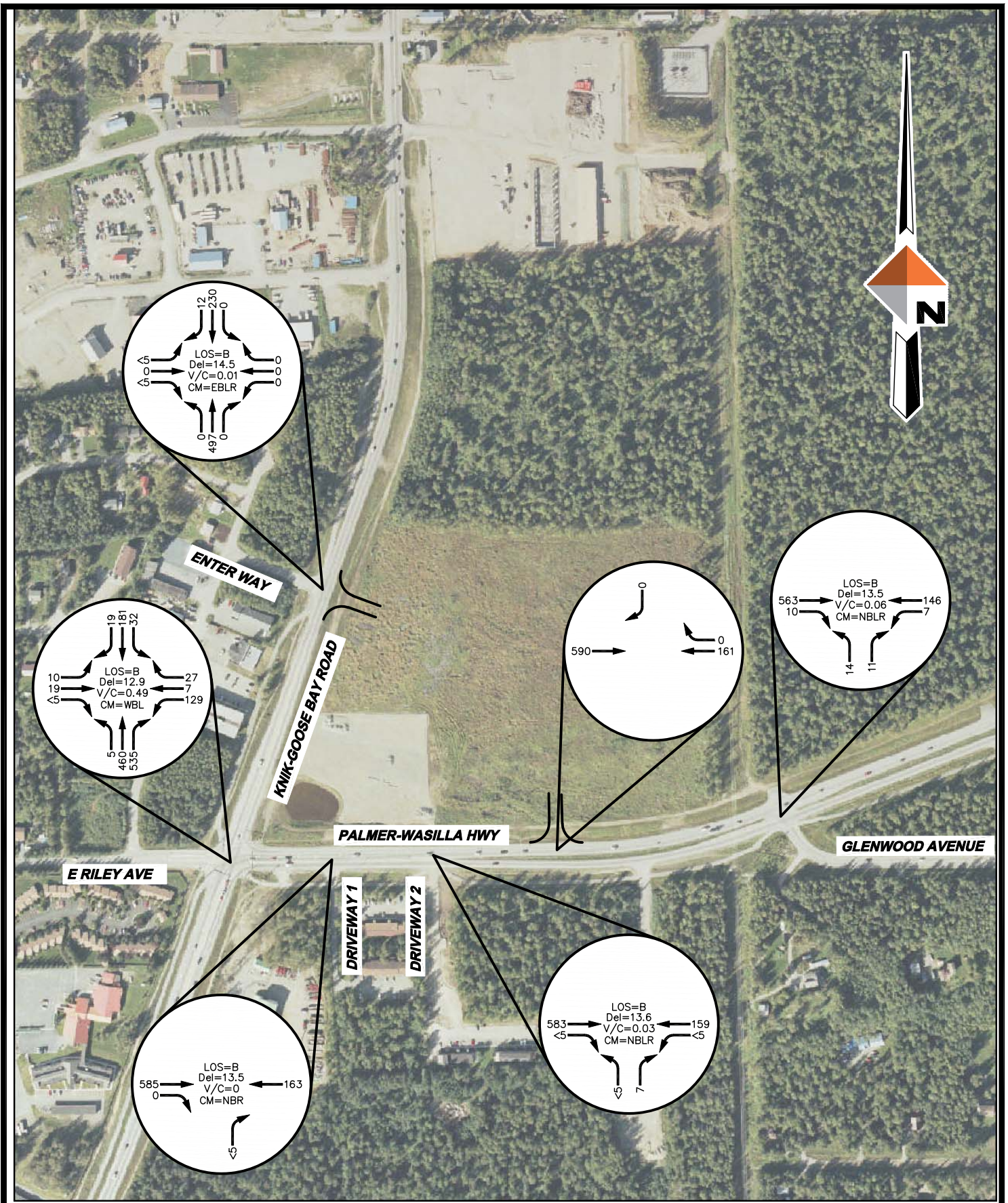
3.0 PROJECTED TRAFFIC

This TIA identifies how the property's accesses currently operate, as well as how they will operate during the construction and design years. The construction year is defined as the year the proposed construction will be completed. The design year is defined by DOT&PF Driveway Regulations as 10 years from the construction completion. For the purpose of this TIA, we assumed that the construction will be completed in 2012, therefore the construction year is 2012 and the design year is 2022.

3.1 Background Traffic

The background condition analysis identifies how the project area's transportation system is expected to operate during the construction and design years without traffic generated by the proposed development. In other words, the background traffic analysis includes the existing traffic in the study area and any expected growth, but does not include the site-generated traffic.

Background traffic for the construction and design years were calculated using a 4.5% growth rate, derived from historical AADT between 2006 and 2009, as published in DOT&PF Annual Traffic Reports. Palmer-Wasilla Highway between Parks Highway and Knik-Goose Bay Road was constructed in 2006. This significantly impacted the study area's AADT. Although growth rates are typically based on 10 years of data, AADT volumes prior to 2006 were excluded from the analysis. Complete calculations are provided in Appendix B. Figures 4 through 7 summarize the construction and design years background traffic volumes at each intersection.

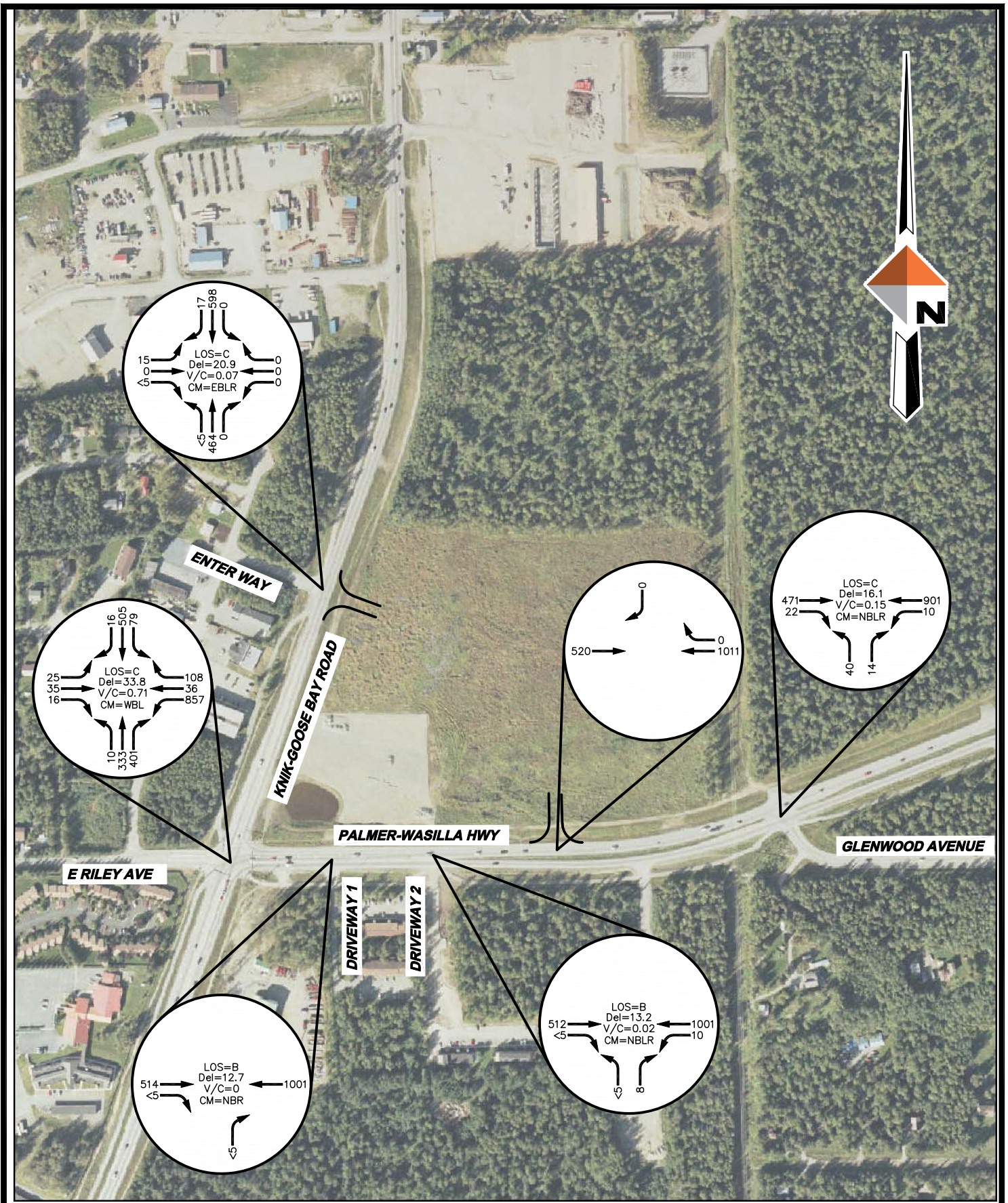


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Construction Year 2012 - A.M. Background Traffic
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 4

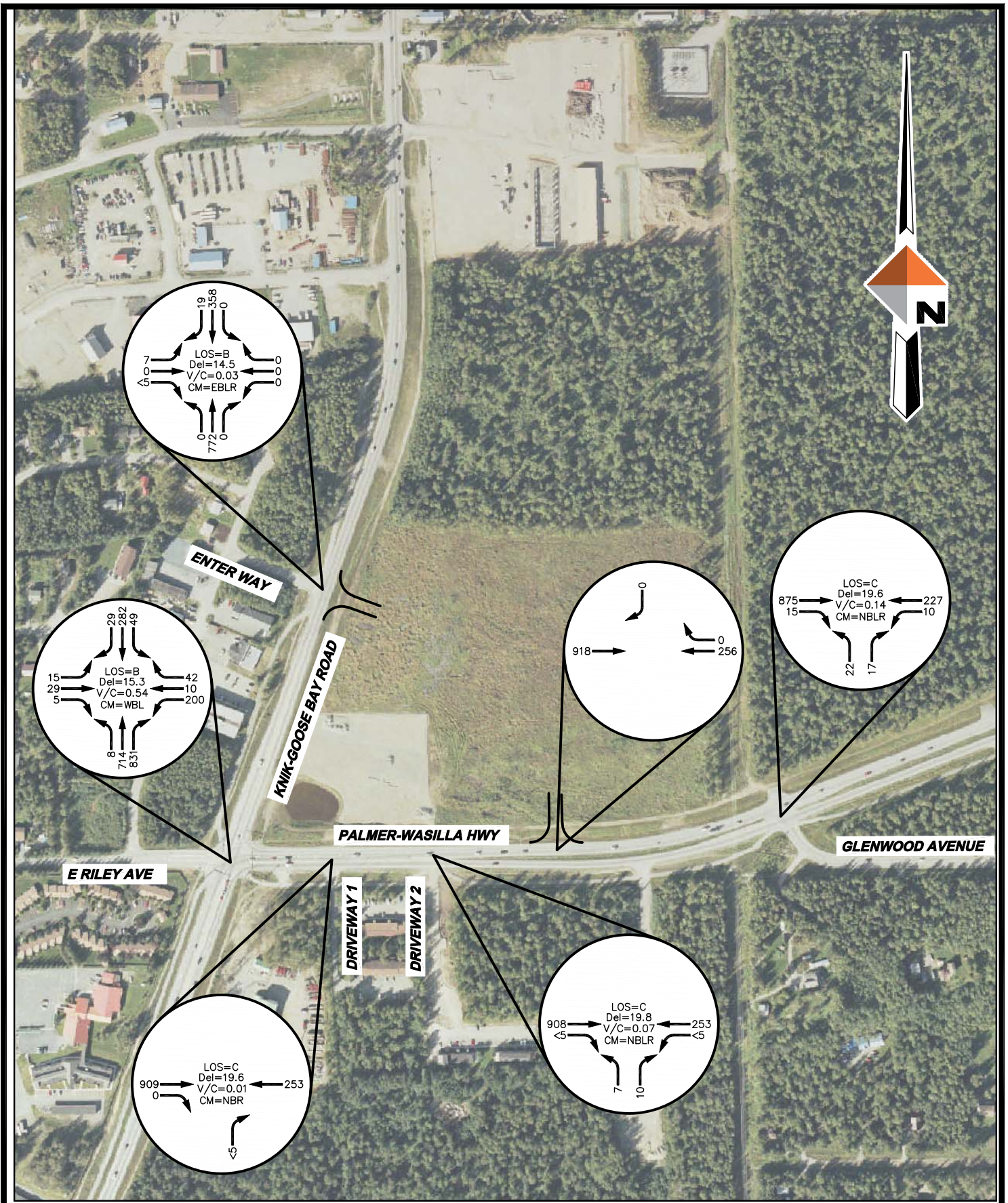


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Construction Year 2012 - P.M. Background Traffic
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 5

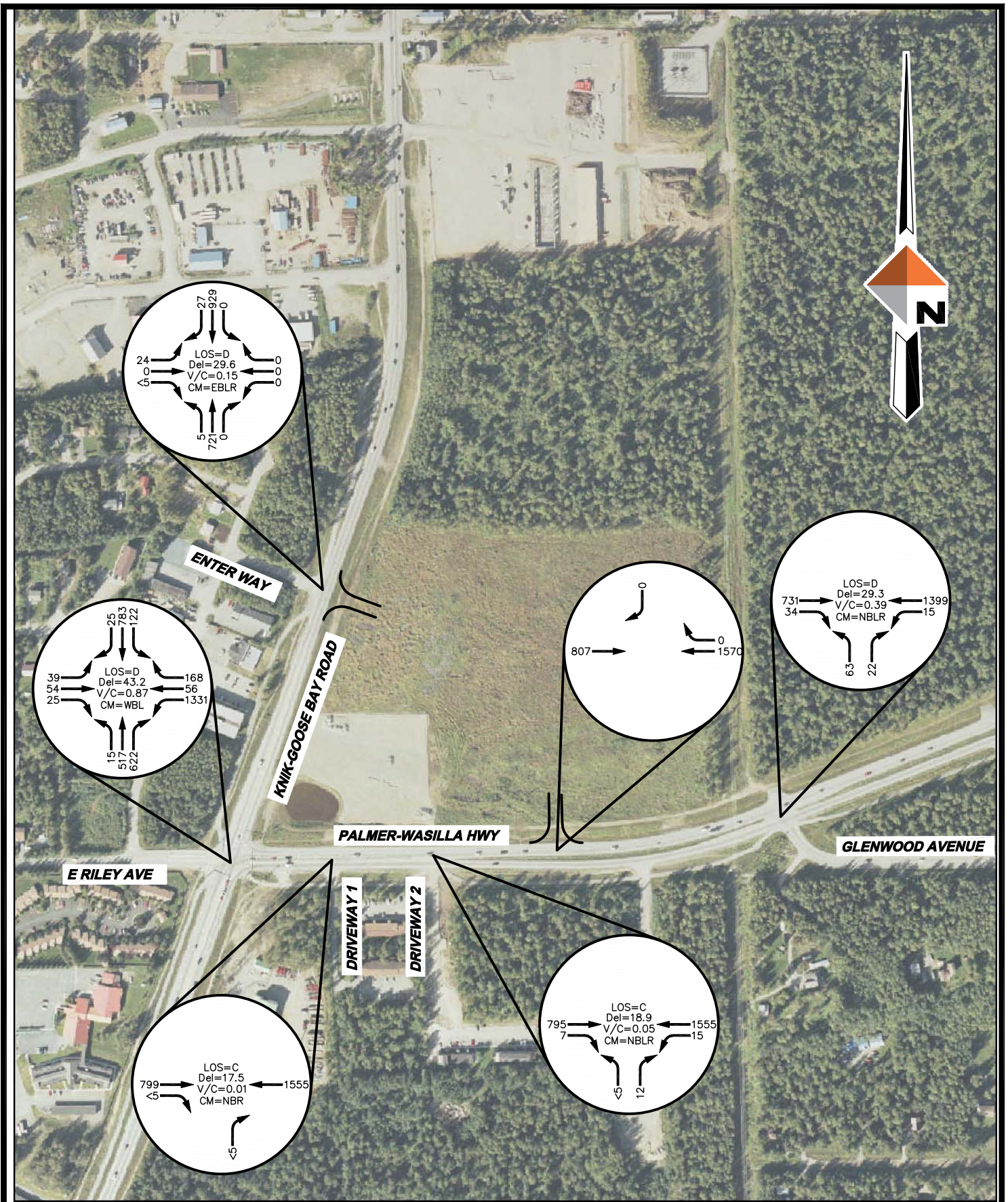


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Design Year 2022 - A.M. Background Traffic
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 6



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Design Year 2022 - P.M. Background Traffic
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 7

3.2 Site Traffic

3.2.1 Trip Generation

The trip generation analysis yields the net new vehicle trips entering and exiting the site and net new vehicles trips on the adjacent roadways and driveways during the a.m. and p.m. peak-hours. Site-generated traffic is generally categorized into four types: new, pass-by, diverted, and internal trips.

New trips are trips that would not have existed within the study area without the proposed development.

Pass-by trips are trips that exist on the roadway immediately adjacent to the site and enter the proposed development because it is on the way to their ultimate trip destination. Due to the nature of the development, the pass-by trips are assumed to be zero.

Internal trips are trips generated by other developments within the project site and only require internal driveways to access the specific development. No internal trips are expected for this development.

Modal split is defined as the percentage of generated trips that use alternative modes of transportation such as public transportation.

The Mat-Su Community Transit (MASCOT) currently does not operate any bus routes adjacent to the subject site.

Trip generation rates for the proposed development are based on the data published in the *ITE Trip Generation Manual*, 7th Edition. Table 1 contains the trip calculation data for the a.m. and p.m. peak hours.

Table 1: Trip Generation

Type of Use	Square Feet/Units	A.M./P.M.	ITE Code/Other	Trip Rate*	Peak Hour 'In' Trips	Peak Hour 'Out' Trips	Peak Hour Total Trips
Medical Office	100,000	P.M.	720	2.82	76	206	282
Medical Office	100,000	A.M.	720	2.30	182	48	230

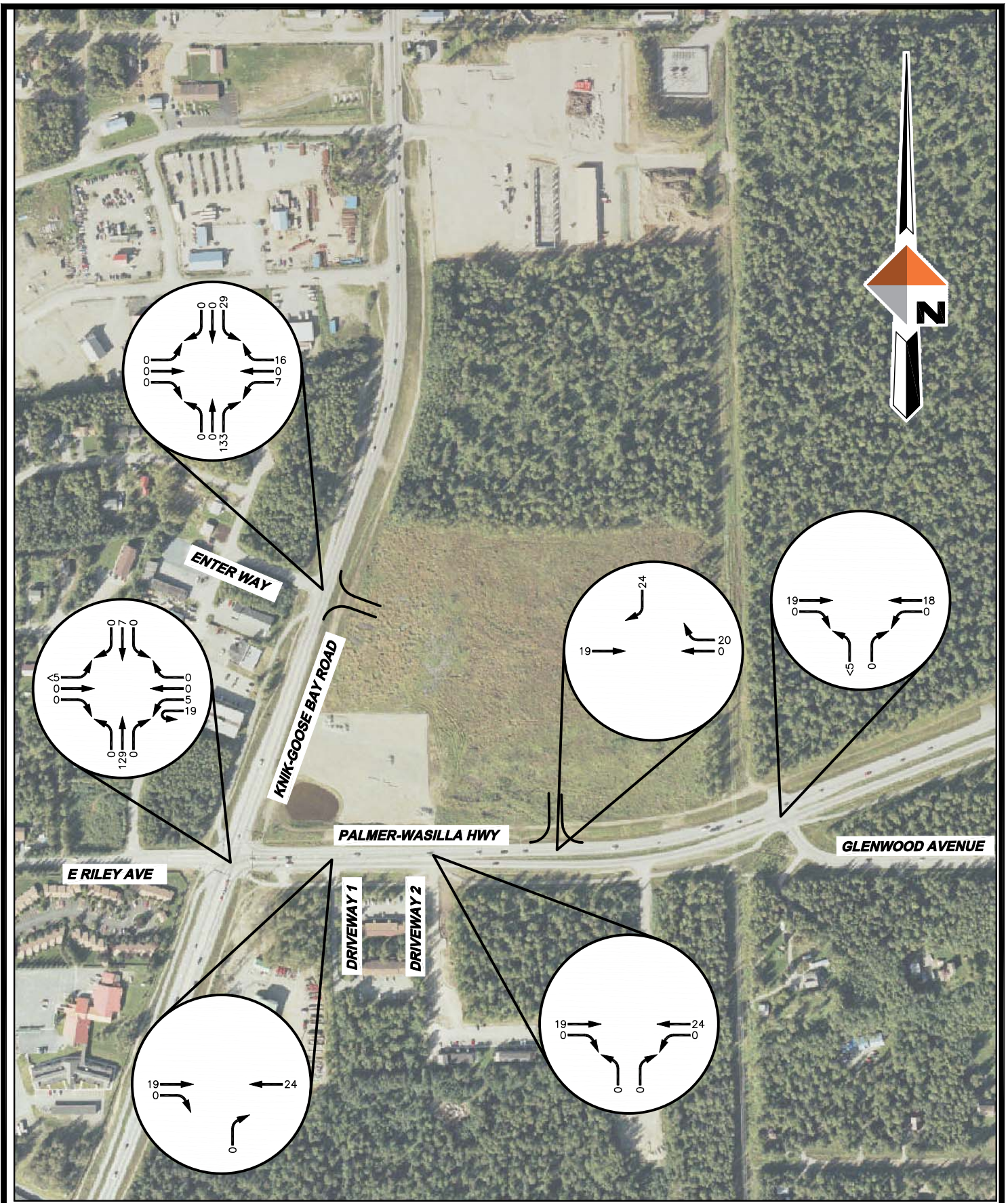
*Trip rate based on fitted curve equation

3.2.2 Trip Distribution and Assignment

The distribution of site-generated trips onto the roadway network within the study area is estimated based on the existing traffic distribution on the adjacent roadways existing land use, and engineering judgment. Two alternate trip distributions were studied. The base distribution has no eastbound left turns into the site access from Palmer-Wasilla Highway. The alternate distribution includes the eastbound left turns from Palmer-Wasilla Highway. The base trip distribution and assignments are shown on Figures 8 and 9.

3.2.3 Total Traffic

The total future traffic is defined as the sum of the background traffic volumes, diverted trips and the net new trips. Figures 10 through 13 summarize the construction and design years total traffic volumes at each intersection.

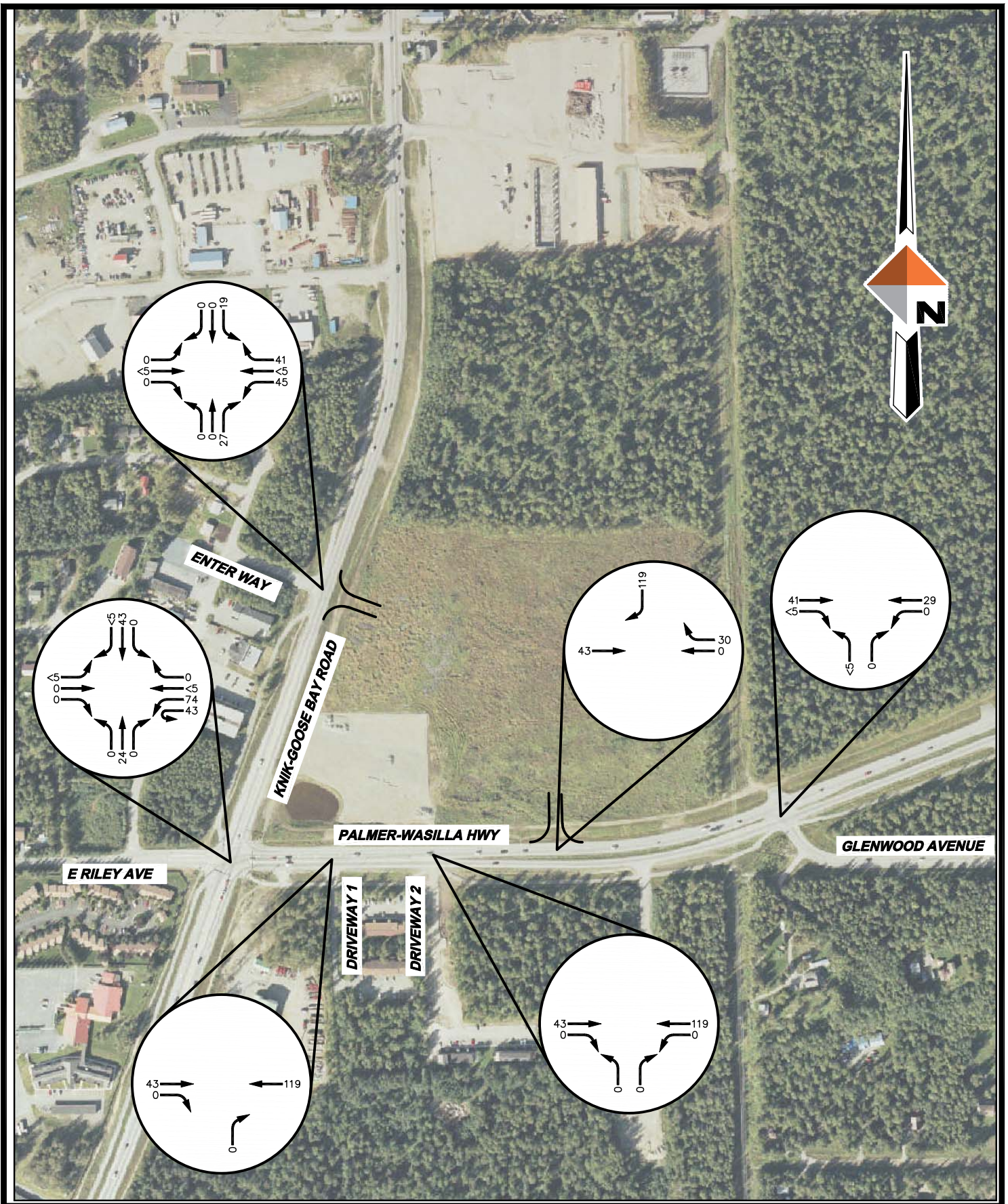


\\dow\cluster1\ANC-Projects\24\60763\01\AutoCAD\Figures.dwg 2011-2-3



100,000 Med. Office Building - A.M. Generated Trips
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 8

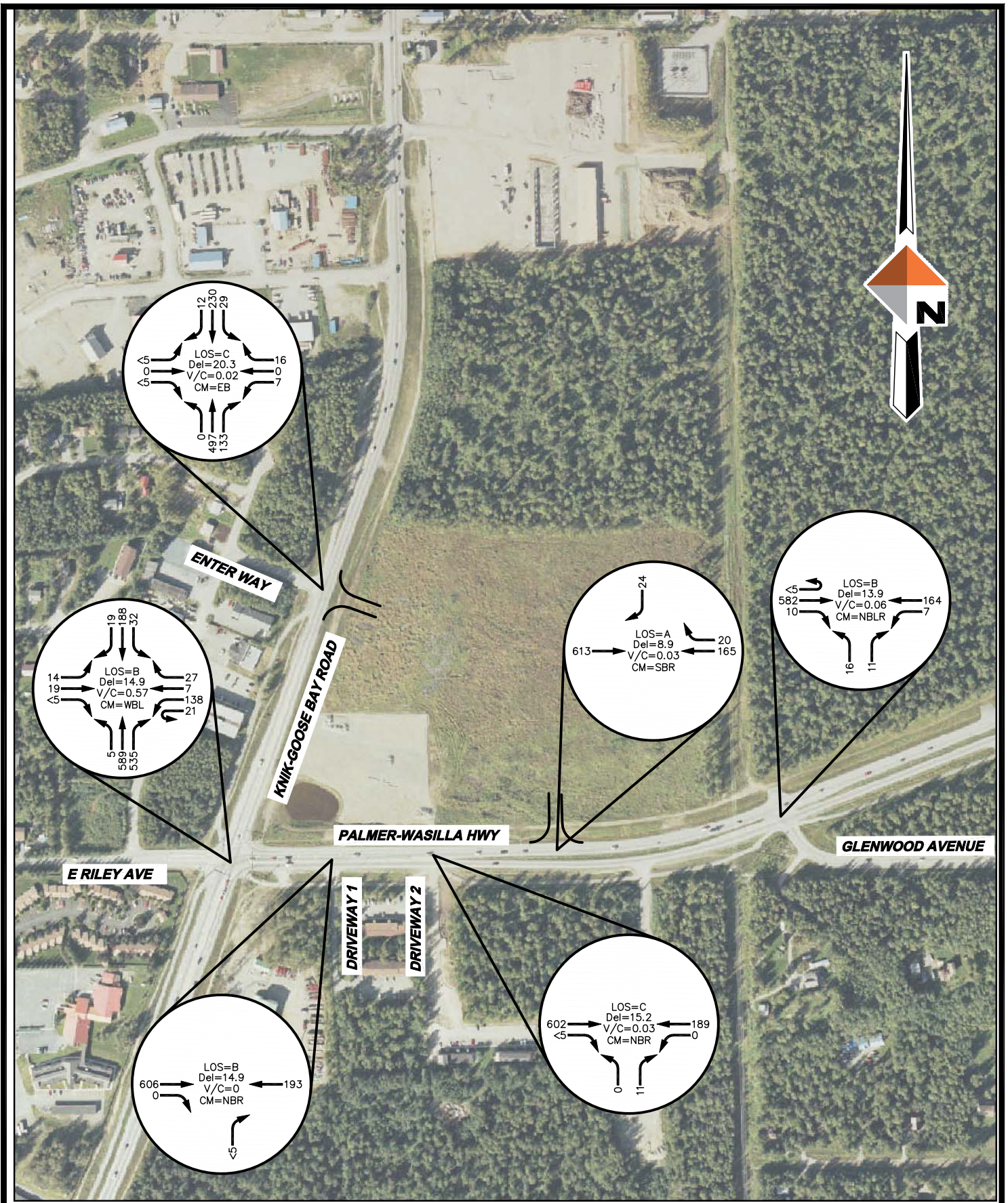


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100,000 SF Med. Office Building - P.M. Generated Trips
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 9

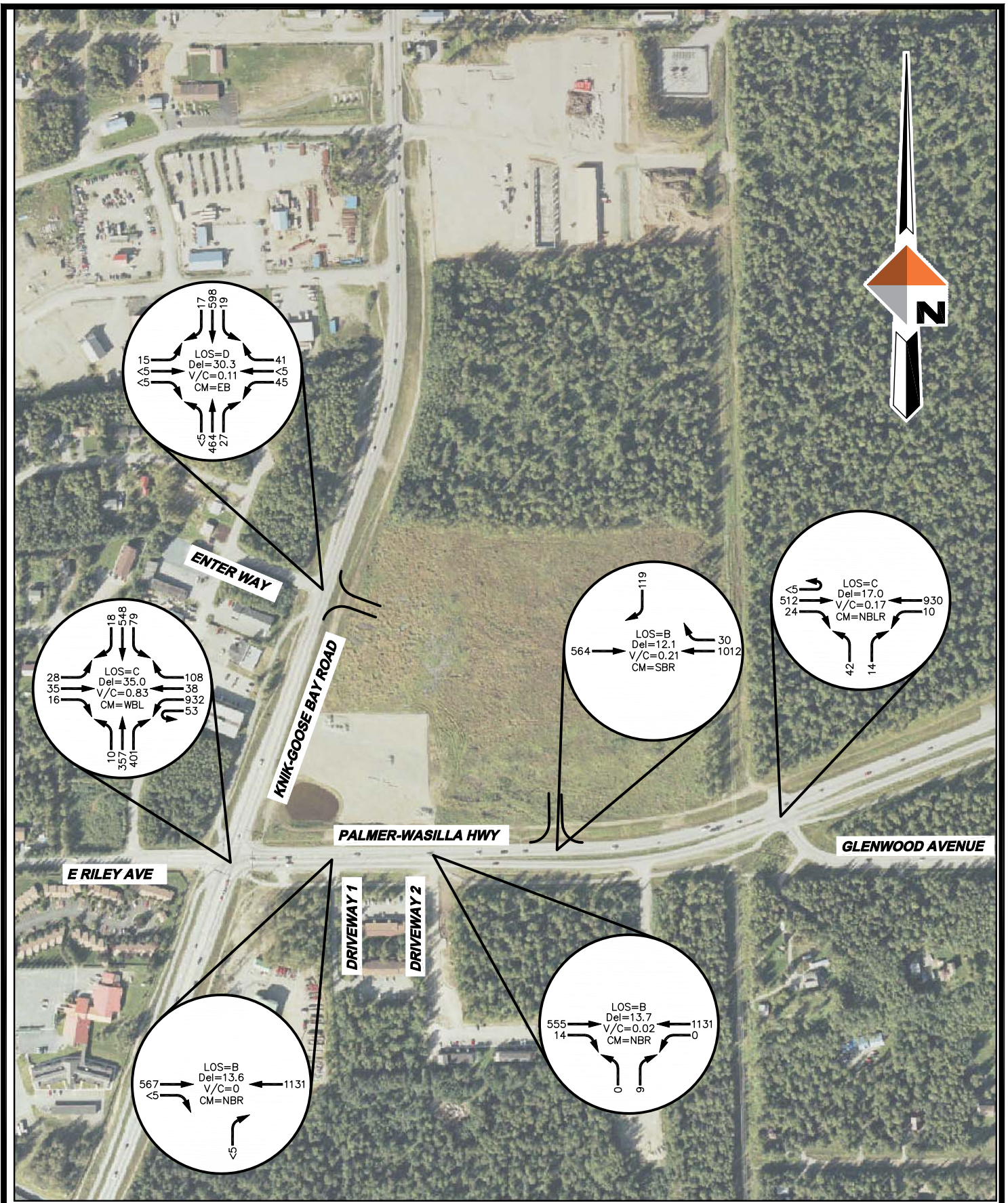


\\dow\cluster1\ANC-Projects\24\60763\01\AutoCAD\Figures.dwg 2011-2-3



Construction Year 2012 - A.M. Total Traffic Volumes
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 10

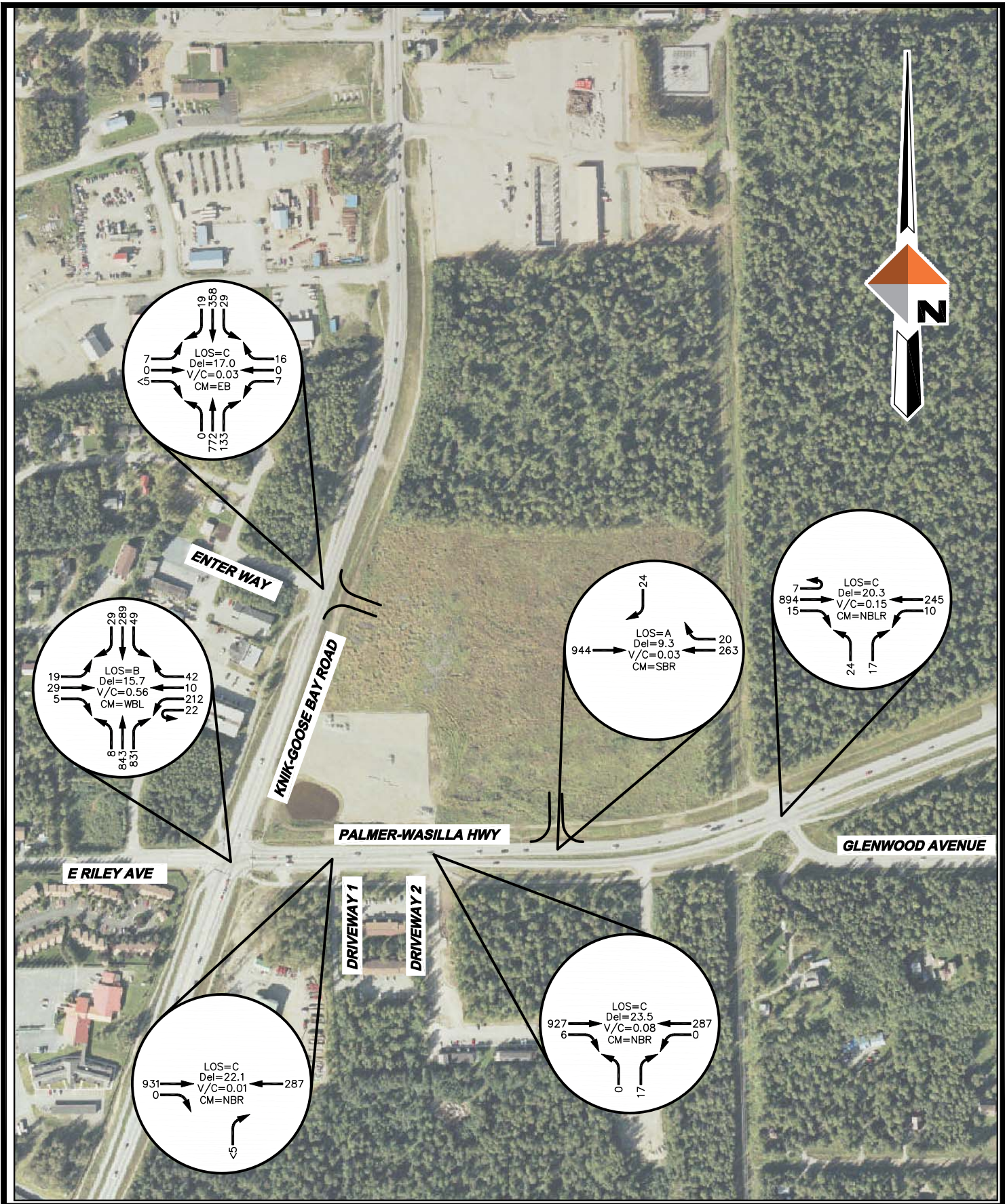


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Construction Year 2012 - P.M. Total Traffic Volumes
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 11

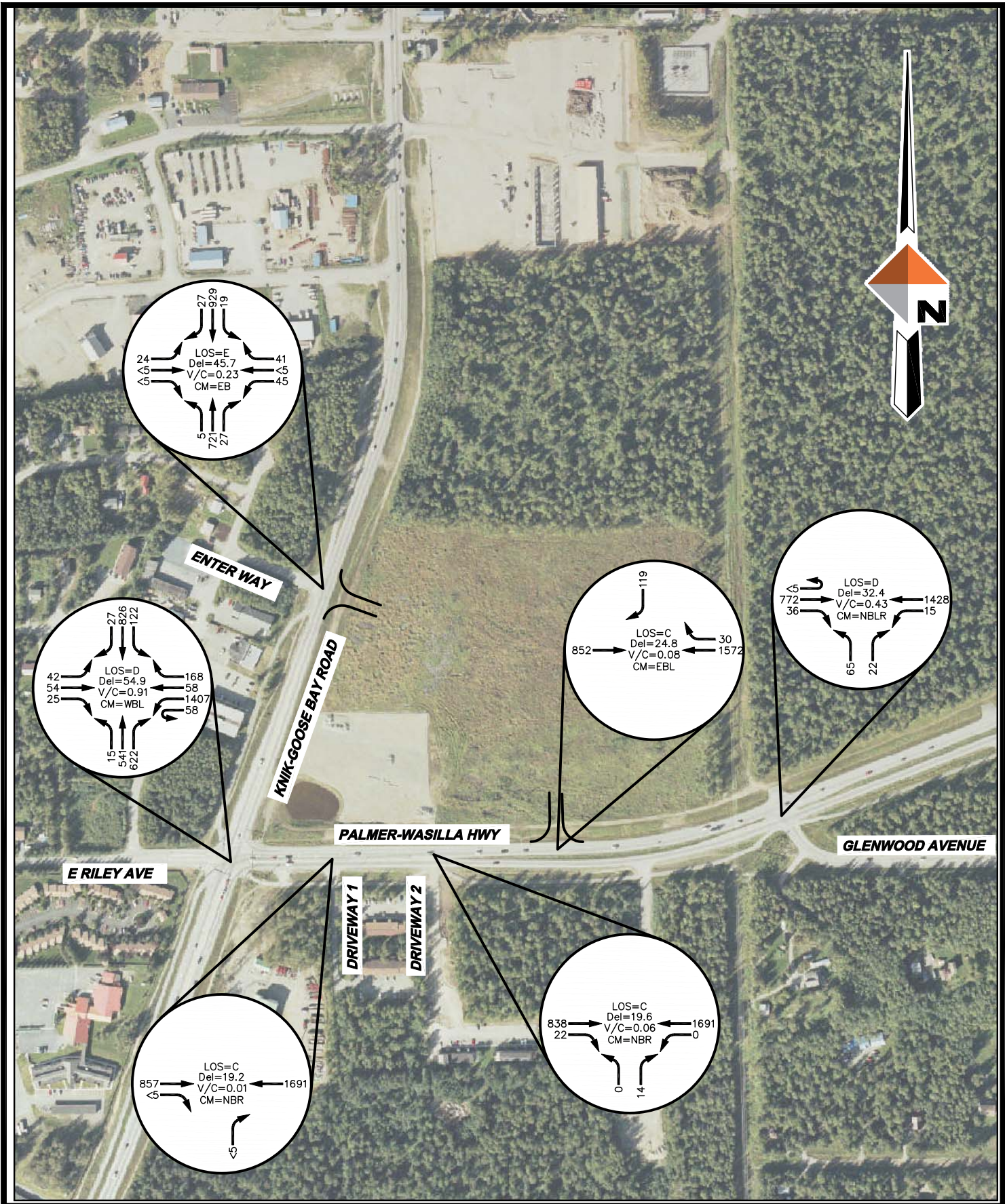


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Design Year 2022 - A.M. Total Traffic Volumes
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 12



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Design Year 2022 - P.M. Total Traffic Volumes
 SCF Valley Medical Complex
 Traffic Impact Analysis
 Wasilla, Alaska

FIGURE 13

4.0 TRAFFIC ANALYSIS

4.1 Capacity Analysis

4.1.1 Methodology

The following software programs were employed to analyze all signalized and unsignalized intersections within the study area:

- Synchro 7.0 for signalized intersections, and
- Highway Capacity Software 2000 (HCS 2000), for unsignalized intersections.

The analysis uses the following measures of effectiveness: LOS, volume-to-capacity ratio (v/c), and the average control delay (sec/veh).

4.1.2 Minimum Level of Service Criteria

DOT&PF's Driveway Design Standards and Regulations (17 AAC 10.070) establishes a minimum acceptable LOS for the development's construction and design years. These standards state that the minimum acceptable LOS at the time of the driveway permit application in both the construction and design year is:

- LOS C, if the LOS on the date of application is LOS C or better; or
- LOS D, if the LOS on the date of application is LOS D or poorer; however, if the LOS is poorer than LOS D, a lower minimum LOS is acceptable if the operation of the highway does not deteriorate more than 10% in terms of delay time or other appropriate measure of effectiveness from the LOS before the development's opening date.

Based on DOT&PF's regulations, if a highway or intersection has an acceptable LOS without traffic generated by the development and an unacceptable LOS with traffic generated by the development, the driveway permit applicant is required to mitigate impacts. Mitigation may include reducing the site generated trips, constructing improvements to the highway or intersection to achieve acceptable LOS, and/or other measures as approved by DOT&PF.

4.1.3 Level of Service Summary

Tables 2 and 3 summarize the LOS and delay conditions for the intersections within the study area. Detailed analysis data from Synchro and HCS 2000 are included in Appendix C.

Table 2: Construction (2012) and Design (2022) Year A.M. Level of Service and Delay Summary

Intersection	2012 A.M. Background		2012 A.M. Traffic Total		2022 A.M. Background		2022 A.M. Traffic Total		Mitigation Needed?
	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	
Enter Way/ Knik-Goose Bay Road	B	14.5	C	20.3	B	14.5	C	17.0	No
Palmer-Wasilla Highway/ Knik-Goose Bay Road	B	12.9	B	14.9	B	15.3	B	15.7	No
Driveway 1/ Palmer-Wasilla Highway	B	13.5	B	14.9	C	19.6	C	22.1	No
Driveway 2/ Palmer-Wasilla Highway	B	13.6	C	15.2	C	19.8	C	23.5	No
Glenwood Avenue/ Palmer-Wasilla Highway	B	13.5	B	13.9	C	19.6	C	20.3	No
Site Entrance/ Palmer-Wasilla Highway	N/A	N/A	A	8.9	N/A	N/A	A	9.3	No

Table 3: Construction (2012) and Design (2022) Year P.M. Level of Service and Delay Summary

Intersection	2012 P.M. Background		2012 P.M. Traffic Total		2022 P.M. Background		2022 P.M. Traffic Total		Mitigation Needed?
	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	
Enter Way/ Knik-Goose Bay Road	C	20.9	D	30.3	D	29.6	E	45.7	Yes
Palmer-Wasilla Highway/ Knik-Goose Bay Road	C	33.8	C	35.0	D	43.2	D	54.9	No
Driveway 1/ Palmer-Wasilla Highway	B	12.7	B	13.6	C	17.5	C	19.2	No
Driveway 2/ Palmer-Wasilla Highway	B	13.2	B	13.7	C	18.9	C	19.6	No
Glenwood Avenue/ Palmer-Wasilla Highway	C	16.1	C	17.0	D	29.3	D	32.4	No
Site Entrance/ Palmer-Wasilla Highway	N/A	N/A	C	15.8	N/A	N/A	B	12.1	No

4.2 MITIGATION ANALYSIS

Based on the data in Tables 2 and 3, the only intersection that requires mitigation is the intersection of Enter Way/Knik-Goose Bay Road. Due to the close proximity of the signal at Palmer-Wasilla Highway, signalized control was not considered.

4.2.1 Enter Way/Knik-Goose Bay Road:

Alternative 1: No Mitigation. This alternative consists of keeping the existing configuration of the intersection.

Alternative 2: Two-Way-Left-Turn Lane. This alternative consists of widening Knik-Goose Bay Road to accommodate a two-way-left-turn lane. This alternative allows this intersection to function at an acceptable LOS in the construction year and design year.

Alternative 3: Turn Lanes. A major road right-turn lane warrant analysis was conducted for this intersection in accordance with Figure 4.23 of NCHRP 279. A northbound right-turn lane is warranted for the construction and design years at this intersection.

A major road left-turn lane warrant analysis was conducted for this intersection in accordance with Exhibit 9-75 of AASHTO 2001. A southbound left-turn lane is warranted for both the construction and design years at this intersection. This alternative results in a LOS D in the construction and design years.

Alternative 4: Eliminate Westbound Left-Turns. This alternative consists of adding a median to the east leg of this intersection that restricts left turns from the site. This alternative results in a LOS B in the construction and design years.

Table 4: Level of Service and Delay Summary of Mitigation Alternatives

Enter Way/Knik-Goose Bay Road				
Mitigation Alternative	2012 PM		2022 PM	
	LOS	Delay	LOS	Delay
No Mitigation	D	30.3	E	45.7
Two-Way-Left-Turn Lane	C	18.3	C	23.7
Turn Lanes	D	26.5	D	33.5
Eliminate Westbound Left Turns	B	12.4	B	11.8

4.2.2 Site Entrance/Palmer-Wasilla Highway

The site access along Palmer-Wasilla Highway will function at an acceptable LOS in all scenarios. A major road right-turn lane warrant analysis was conducted for this intersection in accordance with Figure 4.23 of NCHRP 279. A westbound right-turn taper is warranted for the construction and design years for this intersection. Using NCHRP 479 a right-turn lane is warranted in both the construction and design years.

The new site access to Palmer-Wasilla Highway will create a demand for westbound U-turning traffic at the gap in the median west of the access. This movement will conflict with the existing northbound left turns from the apartment complex. To mitigate this, DOT&PF has indicated that they will require the median from the intersection of Palmer-Wasilla Highway/Knik-Goose Bay Road be extended to the median along Palmer-Wasilla Highway corridor.

This will create a westbound U-turn demand at the Palmer-Wasilla Highway/Knik-Goose Bay Road intersection. The existing configuration does not allow for U-turns. Anticipated updates consist of widening the east leg and relocating the existing signal pole on the southeast corner. Existing traffic from the apartment complex south of Palmer-Wasilla Highway will move to the intersections of Palmer-Wasilla Highway/Knik-Goose Bay Road and Glenwood Avenue/Palmer-Wasilla Highway as U-turns.

The option of adding an eastbound left-turn into the site was analyzed. This option decreased the delay at the Enter Way/Knik-Goose Bay Road intersection by diverting trips to the entrance on Palmer-Wasilla Highway.

4.2.3 Pedestrian Considerations

Knik-Goose Bay Road and Palmer-Wasilla Highway do not have pedestrian facilities on the east and north sides of the roadways, respectively. Pedestrian connections to the right-of-way should be planned as part of this development.

5.0 CONCLUSIONS

The following mitigation alternatives result in an acceptable LOS during the construction and design years.

Enter Way/Knik-Goose Bay Road:

- construct a two-way-left-turn lane along Knik-Goose Bay Road, or
- construct northbound right-turn and southbound left-turn pockets with a median restricting westbound left turns from the site.

Site Access/Palmer-Wasilla Highway:

- construct a westbound right-turn taper, or pocket,
- construct a median to close the gap in the median along Palmer-Wasilla Highway, and
- Construct an optional eastbound left-turn pocket.

Palmer-Wasilla Highway/Knik-Goose Bay Road:

- construct signal upgrades to allow for westbound U-turns.

APPENDIX A

Scoping Meeting Minutes

Traffic Impact Analysis Scoping Meeting

Meeting Date/Time/Location: October 19th, 2010/9:30 a.m./DOT&PF Conference Room

Project Title: Mat-Su South Central Foundation Phase 1

Project Components: 100,000 SF Medical Office Building

Project Location: Northeast corner of Knik Goose Bay (KGB) Road/Palmer-Wasilla Hwy

Consultant: DOWL HKM

Attendees:

- Scott Thomas, DOT&PF Regional Traffic Engineer
- Chris Grgich, DOWL HKM
- LaQuita Chmielowski, DOWL HKM
- Kurt Hulteen, DOWL HKM

Topics:

1. Trip Distribution
 - a. North via KGB Road: 40% Entering/40% Leaving
 - b. South via KGB Road: 18% Entering/18% Leaving
 - c. East via Palmer-Wasilla Highway: 40% Entering/40% Leaving
 - d. West via Enter Way/Riley Avenue: 2% Entering/2% Exiting
2. Project Timeframe
 - a. Construction Completion and Acceptance: September 2012
3. Model Assumptions
 - a. Project year: 2012
 - b. Design Year: 2022
4. Study Area
 - a. Area Boundaries
 - i. North: Property boundary
 - ii. South: Palmer-Wasilla Highway
 - iii. East: KGB Road
 - iv. West: Glenwood Avenue
 - b. Critical Roadways and Intersections to be included
 - i. Palmer-Wasilla Highway/KGB Road Signalized Intersection
 - ii. Glenwood Avenue/Palmer-Wasilla Highway
 - iii. Enter Way/KGB Road

- iv. Any other site proposed site access points
- 5. Existing Data
 - a. DOT&PF Previous letters and direction from past Draft TIA
- 6. Traffic Growth Rates
 - a. Assume 2.0%
- 7. Trip Generation
 - a. New Trips – ITE generation rates, Volume 7.0,
 - i. Medical Office Land Use (ITE Land Use Code 720)
 - ii. Trip rate calculated based on Logarithmic Correlation
 - iii. Trip Rate = 2.82 trips/1000SF of leasable area
 - iv. Total New Trips during PM Peak Hour: 282
 - v. Direction Split: Entering: 76, Exiting: 206
 - b. Pass-by Trips – 0%
 - c. Diverted Trips – 0%
 - d. Internal Trips – 0%
- 8. Other Developments to be included in TIA
 - a. NA
- 9. Construction Projects Within Study Area
 - a. NA
- 10. Circulation Issues
 - a. Roadway
 - i. Due to the large acreage of the property, an access route to Glenwood to the east should be preserved. Because we're not platting/sub-dividing the property, this does not need to be in the form of a common access easement, but should be considered in the preliminary site plan. DOT&PF also foresees a future signal on the north west corner of the property, and that it would eventually become the main access point for the full site. At this time, preserve access to the north for future connections. These access points will not be included in this TIA.
 - ii. A full access driveway to KGB can be considered (i.e. no requirement from the DOT&PF to build a median on KGB), for the development of

the 100K Medical Building. This will relieve the U-Turn demand on the KGB/Palmer-Wasilla Highway signal.

- iii. A right-in/right-out only driveway would be allowed in the current shared use easement to KGB. However, if this easement is not used, the property owners will need to establish an agreement of shared access for other driveways. No re-platting will be required.
- iv. All un-signalized accesses to state ROW will require turn-lane warrants.
- v. The DOT&PF would prefer a single access to Palmer-Wasilla along the southern property frontage. This access should be located outside of the dual westbound left-turn lanes, and leave enough corner clearance for a future signal at Glenwood. If the site plan were adjusted to allow for this single access point, the DOT&PF foresees allowing a left-turn-in from the Palmer-Wasilla Highway.
- vi. Median work on Palmer-Wasilla Highway will be required per previous DOT&PF direction. This work will connect the existing median on the highway to the median at the signal, and will restrict left-turns entering and exiting the highway. The neighboring apartment complex has been alerted to the possibility of this work in an August 2007 letter.
- vii. Current signalized intersection at Palmer-Wasilla Highway/KGB road is not large enough to allow westbound U-turn movements. The widening will likely require the relocation of the signal pole on the southeast corner. Assume new pole and signal hardware at this location.
- viii. State recommends building setbacks along KGB to allow for the construction of a future couplet to the north. This will likely occur north of the site, and the state is not asking for any easements at this time. However they do not want to have any option precluded. Parking along the corridor, as currently sketched in our bubble diagram, does allow for this future work.

a. Transit

- i. NA

- b. Pedestrian/Bicycle
 - i. NA
- 2. Meeting notes - to be provided by consultant to the Traffic Staff within 5 days of the meeting for Traffic review and comment.

McGill, Adam

From: Chmielowski, LaQuita
Sent: Monday, November 01, 2010 8:55 PM
To: Grgich, Christopher; Hulteen, Kurt
Subject: FW: Mat-Su SCF Draft TIA Scoping Meeting Minutes

Follow Up Flag: Follow up
Flag Status: Completed

Hi, Chris and Kurt,

FYI – Looks like Scott has a few changes to the meeting minutes. It appears that some of his comments may affect the current site plan.

Thanks
LaQuita

From: Thomas, Scott E (DOT) [<mailto:scott.thomas@alaska.gov>]
Sent: Monday, November 01, 2010 3:20 PM
To: Chmielowski, LaQuita
Subject: RE: Mat-Su SCF Draft TIA Scoping Meeting Minutes

LaQuita

Some changes/ comments:

I don't remember agreeing on trip distributions. I would need to see that it is similar to existing demands in the TIA. I expect more heading to the Parks Hwy via KGB.

Traffic growth rates: Used to be 4%. I'll need a newer ISER number or % from the Borough as per their LRTP modeling assumptions.

7. 8.

Trip Generation is for this phase of development. Should the owner add more to the site at a later date, then we would want an updated TIA if the trips are large enough.

10.ii. While full movements on KGB will be considered at this time, it would be interim. DOT/PF may want to restrict LT's in the future, depending on the couplet final configuration and on preserving and eventually seeing the construction of an option to access a signal/intersection to the north boundary of the property.

10.iv. Turn lane warrant "analysis".

10.v. I don't foresee left turns considered by DOT/PF from the PW Hwy unless all other access points suffer LOS (queues, delay for large traffic #'s) without it. Yes, it would have to fit between Glenwood and KGB turn lanes.

10.vi. Medina work is required if there is to be a driveway that is too close to this break and causes unintentional demand for eastbound left turns onto the property or off of the property.

Transit. Good point. Want valley transit to be allowed for in this plan, first onsite. Offsite on the roadway in pullouts will only be per MSB agreement/request to DOT.

Ped/Bike = access routes to signal need to be planned for.

Scott

From: Chmielowski, LaQuita [<mailto:lchmielowski@dowlhkm.com>]
Sent: Wednesday, October 27, 2010 12:31 PM
To: Thomas, Scott E (DOT)
Cc: Reception; Grgich, Christopher; Hulteen, Kurt
Subject: Mat-Su SCF Draft TIA Scoping Meeting Minutes

Hello, Scott.

I have appended the draft scoping meeting minutes from our meeting with you to discuss the Mat-Su SCF project. Please contact Chris or me if you have any questions or concerns.

Thank you,
LaQuita
D60715

LaQuita M. Chmielowski, P.E., LEED® AP
Civil Engineer



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APPENDIX B

Traffic Volume Documentation

Period Starting	Northbound (to Wasilla)	Southbound (away from Wasilla)	Both Directions	Hourly Total at Start Interval	Distribution (N/S)	
11:00 AM	94	94	81	175	707	54% 46%
11:15 AM	92	92	80	172	717	53% 47%
11:30 AM	94	94	98	192	759	49% 51%
11:45 AM	110	110	58	168	737	65% 35%
12:00 PM	97	97	88	185	782	52% 48%
12:15 PM	116	116	98	214	814	54% 46%
12:30 PM	102	102	68	170	775	60% 40%
12:45 PM	118	118	95	213	784	55% 45%
1:00 PM	115	115	102	217	764	53% 47%
1:15 PM	89	89	86	175	755	51% 49%
1:30 PM	97	97	82	179	795	54% 46%
1:45 PM	95	95	98	193	836	49% 51%
2:00 PM	115	115	93	208	876	55% 45%
2:15 PM	109	109	106	215	851	51% 49%
2:30 PM	94	94	126	220	843	43% 57%
2:45 PM	123	123	110	233	829	53% 47%
3:00 PM	92	92	91	183	811	50% 50%
3:15 PM	103	103	104	207	852	50% 50%
3:30 PM	102	102	104	206	885	50% 50%
3:45 PM	104	104	111	215	922	48% 52%
4:00 PM	96	96	128	224	946	43% 57%
4:15 PM	102	102	138	240	975	43% 58%
4:30 PM	120	120	123	243	976	49% 51%
4:45 PM	116	116	123	239	975	49% 51%
5:00 PM	100	100	153	253	1011	40% 60%
5:15 PM	91	91	150	241	1002	38% 62%
5:30 PM	97	97	145	242	994	40% 60%
5:45 PM	123	123	152	275	962	45% 55%
6:00 PM	97	97	147	244	897	40% 60%
6:15 PM	90	90	143	233	839	39% 61%
6:30 PM	75	75	135	210	780	36% 64%
6:45 PM	83	83	127	210	725	40% 60%
7:00 PM	77	77	109	186	656	41% 59%
7:15 PM	73	73	101	174	617	42% 58%
7:30 PM	72	72	83	155	598	46% 54%
7:45 PM	57	57	84	141	568	40% 60%
8:00 PM	56	56	91	147	537	38% 62%
8:15 PM	60	60	95	155	479	39% 61%
8:30 PM	42	42	83	125	417	34% 66%
8:45 PM	45	45	65	110	382	41% 59%
9:00 PM	39	39	50	89	342	44% 56%
9:15 PM	29	29	64	93	317	31% 69%
9:30 PM	33	33	57	90	293	37% 63%
9:45 PM	27	27	43	70	243	39% 61%
10:00 PM	29	29	35	64	212	45% 55%
10:15 PM	14	14	55	69	192	20% 80%
10:30 PM	18	18	22	40	151	45% 55%
10:45 PM	12	12	27	39	141	31% 69%
11:00 PM	14	14	30	44	129	32% 68%
11:15 PM	15	15	13	28	108	54% 46%
11:30 PM	11	11	19	30	95	37% 63%
11:45 PM	12	12	15	27	74	44% 56%
12:00 AM	9	9	14	23	61	39% 61%
12:15 AM	7	7	8	15	56	47% 53%
12:30 AM	6	6	3	9	53	67% 33%
12:45 AM	2	2	12	14	51	14% 86%
1:00 AM	5	5	13	18	42	28% 72%
1:15 AM	4	4	8	12	28	33% 67%
1:30 AM	3	3	4	7	21	43% 57%
1:45 AM	4	4	1	5	18	80% 20%

2:00 AM	1	3	4	20	25%	75%
2:15 AM	1	4	5	18	20%	80%
2:30 AM	2	2	4	15	50%	50%
2:45 AM	1	6	7	13	14%	86%
3:00 AM	1	1	2	11	50%	50%
3:15 AM	1	1	2	13	50%	50%
3:30 AM	1	1	2	16	50%	50%
3:45 AM	2	3	5	22	40%	60%
4:00 AM	2	2	4	28	50%	50%
4:15 AM	1	4	5	39	20%	80%
4:30 AM	4	4	8	52	50%	50%
4:45 AM	3	8	11	69	27%	73%
5:00 AM	10	5	15	97	67%	33%
5:15 AM	11	7	18	145	61%	39%
5:30 AM	11	14	25	180	44%	56%
5:45 AM	24	15	39	219	62%	38%
6:00 AM	39	24	63	262	62%	38%
6:15 AM	26	27	53	327	49%	51%
6:30 AM	54	10	64	474	84%	16%
6:45 AM	60	22	82	574	73%	27%
7:00 AM	108	20	128	638	84%	16%
7:15 AM	151	49	200	667	76%	25%
7:30 AM	93	71	164	606	57%	43%
7:45 AM	100	46	146	588	68%	32%
8:00 AM	111	46	157	616	71%	29%
8:15 AM	99	40	139	650	71%	29%
8:30 AM	98	48	146	647	67%	33%
8:45 AM	110	64	174	674	63%	37%
9:00 AM	123	68	191	651	64%	36%
9:15 AM	77	59	136	656	57%	43%
9:30 AM	94	79	173	699	54%	46%
9:45 AM	95	56	151	616	63%	37%
10:00 AM	120	76	196		61%	39%
10:15 AM	105	74	179		59%	41%
10:30 AM	85	5	90		94%	6%

SCF VALLEY MEDICAL COMPLEX PHASE 1 TIA

RAW TRAFFIC DATA

Data Collected By:
DOWL HKM
4041 B Street, Anchorage, AK
(907) 562-2000

File Name : PW HWY_KGB TOTAL AM
Site Code : 00000000
Start Date : 12/01/2010
Page No : 1

Groups Printed- 1 - Unshifted

Start Time	KGB From North					PW HWY From East					KGB From South					PW HWY From West					Int. Total
	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00 AM	5	0	3	0	8	4	0	19	0	23	125	1	2	0	128	0	0	3	0	3	162
07:15 AM	2	0	7	0	9	11	2	24	0	37	121	0	0	0	121	3	4	1	0	8	175
07:30 AM	4	0	8	0	12	5	3	28	0	36	127	0	2	0	129	0	6	2	0	8	185
07:45 AM	7	0	8	0	15	4	1	32	0	37	137	0	1	0	138	0	5	5	0	10	200
Total	18	0	26	0	44	24	6	103	0	133	510	1	5	0	516	3	15	11	0	29	722
08:00 AM	4	0	6	0	10	5	0	34	0	39	105	2	2	0	109	0	2	1	0	3	161
08:15 AM	3	0	5	0	8	7	2	37	0	46	112	0	4	7	123	1	2	3	0	6	183
08:30 AM	3	0	5	0	8	7	3	42	0	52	99	0	2	0	101	0	4	4	0	8	169
08:45 AM	5	0	3	0	8	17	1	35	0	53	96	0	3	0	99	1	1	1	0	3	163
Total	15	0	19	0	34	36	6	148	0	190	412	2	11	7	432	2	9	9	0	20	676
Grand Total	33	0	45	0	78	60	12	251	0	323	922	3	16	7	948	5	24	20	0	49	1398
Apprch %	42.3	0.0	57.7	0.0		18.6	3.7	77.7	0.0		97.3	0.3	1.7	0.7		10.2	49.0	40.8	0.0		
Total %	2.4	0.0	3.2	0.0	5.6	4.3	0.9	18.0	0.0	23.1	66.0	0.2	1.1	0.5	67.8	0.4	1.7	1.4	0.0	3.5	

SCF VALLEY MEDICAL COMPLEX PHASE 1 TIA

RAW TRAFFIC DATA

Data Collected By:
DOWL HKM
4041 B Street, Anchorage, AK
(907) 562-2000

File Name : PW HWY_KGB TOTAL PM
Site Code : 00000000
Start Date : 12/01/2010
Page No : 1

Groups Printed- 1 - Unshifted

Start Time	Knik-Goose Bay Road From North					Palmer-Wasilla Hwy From East					Knik-Goose Bay Road From South					Palmer-Wasilla Hwy From West					Int. Total
	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
04:00 PM	6	0	26	0	32	26	6	124	1	157	103	0	1	0	104	5	4	12	0	21	314
04:15 PM	5	0	17	0	22	23	6	151	0	180	68	0	2	3	73	2	5	11	0	18	293
04:30 PM	2	0	20	0	22	14	4	163	0	181	109	0	4	0	113	0	6	5	0	11	327
04:45 PM	2	0	14	0	16	35	5	223	0	263	86	0	2	0	88	2	10	7	0	19	386
Total	15	0	77	0	92	98	21	661	1	781	366	0	9	3	378	9	25	35	0	69	1320
05:00 PM	6	0	18	0	24	29	17	208	0	254	103	0	3	0	106	4	9	4	0	17	401
05:15 PM	5	0	20	0	25	21	7	191	0	219	69	1	0	0	70	9	7	7	0	23	337
05:30 PM	2	1	15	0	18	10	2	162	0	174	65	0	0	0	65	2	7	5	0	14	271
05:45 PM	1	0	15	0	16	29	2	186	0	217	76	0	0	0	76	3	5	3	0	11	320
Total	14	1	68	0	83	89	28	747	0	864	313	1	3	0	317	18	28	19	0	65	1329
Grand Total	29	1	145	0	175	187	49	1408	1	1645	679	1	12	3	695	27	53	54	0	134	2649
Apprch %	16.6	0.6	82.9	0.0		11.4	3.0	85.6	0.1		97.7	0.1	1.7	0.4		20.1	39.6	40.3	0.0		
Total %	1.1	0.0	5.5	0.0	6.6	7.1	1.8	53.2	0.0	62.1	25.6	0.0	0.5	0.1	26.2	1.0	2.0	2.0	0.0	5.1	

SCF VALLEY MEDICAL COMPLEX PHASE 1 TIA

RAW TRAFFIC DATA

Data Collected By:
DOWL HKM
4041 B Street, Anchorage, AK
(907) 562-2000

File Name : PWHWY_GLENNWOOD AM
Site Code : 00000000
Start Date : 12/01/2010
Page No : 1

Groups Printed- Unshifted

Start Time	GLENWOOD DRIVE From North					PALMER WASILLA HWY From East					GLENWOOD DRIVE From South					PALMER WASILLA HWY From West					Int. Total
	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00 AM	0	0	0	0	0	0	23	1	0	24	0	0	3	0	3	2	135	0	0	137	164
07:15 AM	0	0	0	0	0	0	31	3	0	34	2	0	7	0	9	1	137	0	0	138	181
07:30 AM	0	0	0	0	0	0	31	2	0	33	2	0	2	0	4	7	136	0	0	143	180
07:45 AM	0	0	0	0	0	0	34	1	0	35	2	0	2	0	4	0	126	0	0	126	165
Total	0	0	0	0	0	0	119	7	0	126	6	0	14	0	20	10	534	0	0	544	690
08:00 AM	0	0	0	0	0	0	38	0	0	38	4	0	2	0	6	1	117	0	0	118	162
08:15 AM	0	0	0	0	0	0	37	1	0	38	2	0	6	0	8	2	110	0	0	112	158
08:30 AM	0	0	0	0	0	0	38	1	0	39	1	0	6	0	7	0	110	0	1	111	157
08:45 AM	0	0	0	0	0	0	45	2	0	47	6	0	6	0	12	0	107	0	0	107	166
Total	0	0	0	0	0	0	158	4	0	162	13	0	20	0	33	3	444	0	1	448	643
Grand Total	0	0	0	0	0	0	277	11	0	288	19	0	34	0	53	13	978	0	1	992	1333
Apprch %	0.0	0.0	0.0	0.0		0.0	96.2	3.8	0.0		35.8	0.0	64.2	0.0		1.3	98.6	0.0	0.1		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	20.8	0.8	0.0	21.6	1.4	0.0	2.6	0.0	4.0	1.0	73.4	0.0	0.1	74.4	

SCF VALLEY MEDICAL COMPLEX PHASE 1 TIA

RAW TRAFFIC DATA

Data Collected By:
DOWL HKM
4041 B Street, Anchorage, AK
(907) 562-2000

File Name : PW HWY_GLENNWOOD PM
Site Code : 00000000
Start Date : 12/01/2010
Page No : 1

Groups Printed- Unshifted

Start Time	GLENWOOD DRIVE From North					PALMER WASILLA HWY From East					GLENWOOD DRIVE From South					PALMER WASILLA HWY From West					Int. Total
	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
04:00 PM	0	0	0	0	0	0	128	2	0	130	1	0	6	0	7	7	100	0	0	107	244
04:15 PM	0	0	0	0	0	0	175	3	0	178	1	0	10	0	11	3	108	0	0	111	300
04:30 PM	0	0	0	0	0	0	179	1	0	180	2	0	5	0	7	3	108	0	0	111	298
04:45 PM	0	0	0	0	0	0	190	2	0	192	5	0	8	0	13	6	104	0	0	110	315
Total	0	0	0	0	0	0	672	8	0	680	9	0	29	0	38	19	420	0	0	439	1157
05:00 PM	0	0	0	0	0	0	225	3	0	228	3	0	6	0	9	7	107	0	0	114	351
05:15 PM	0	0	0	0	0	0	231	3	0	234	3	0	18	0	21	4	112	0	1	117	372
05:30 PM	0	0	0	0	0	0	176	4	0	180	5	0	11	0	16	5	78	0	0	83	279
05:45 PM	0	0	0	0	0	0	210	3	0	213	3	0	5	0	8	2	97	0	0	99	320
Total	0	0	0	0	0	0	842	13	0	855	14	0	40	0	54	18	394	0	1	413	1322
Grand Total	0	0	0	0	0	0	1514	21	0	1535	23	0	69	0	92	37	814	0	1	852	2479
Apprch %	0.0	0.0	0.0	0.0		0.0	98.6	1.4	0.0		25.0	0.0	75.0	0.0		4.3	95.5	0.0	0.1		
Total %	0.0	0.0	0.0	0.0	0.0	0.0	61.1	0.8	0.0	61.9	0.9	0.0	2.8	0.0	3.7	1.5	32.8	0.0	0.0	34.4	

SCF VALLEY MEDICAL COMPLEX PHASE 1 TIA

RAW TRAFFIC DATA

Data Collected By:
DOWL HKM
4041 B Street, Anchorage, AK
(907) 562-2000

File Name : KGN_ENTER WAY AM
Site Code : 00000000
Start Date : 12/01/2010
Page No : 1

Groups Printed- Unshifted

Start Time	KGB From North					ENTER W From East					KGB From South					ENTER W From West					Int. Total
	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
07:00 AM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	1	0	2	0	3	6
07:15 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1	0	2	0	3	4
*** BREAK ***																					
07:45 AM	5	0	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5
Total	9	0	0	0	9	0	0	0	0	0	0	0	0	0	0	2	0	4	0	6	15
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
08:15 AM	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	8
08:30 AM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	4
08:45 AM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	6
Total	8	0	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	12	0	12	20
Grand Total	17	0	0	0	17	0	0	0	0	0	0	0	0	0	0	2	0	16	0	18	35
Apprch %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		11.1	0.0	88.9	0.0		
Total %	48.6	0.0	0.0	0.0	48.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.7	0.0	45.7	0.0	51.4	

SCF VALLEY MEDICAL COMPLEX PHASE 1 TIA

RAW TRAFFIC DATA

Data Collected By:
 DOWL HKM
 4041 B Street, Anchorage, AK
 (907) 562-2000

File Name : KGB_ENTER WAY PM
 Site Code : 00000000
 Start Date : 12/01/2010
 Page No : 1

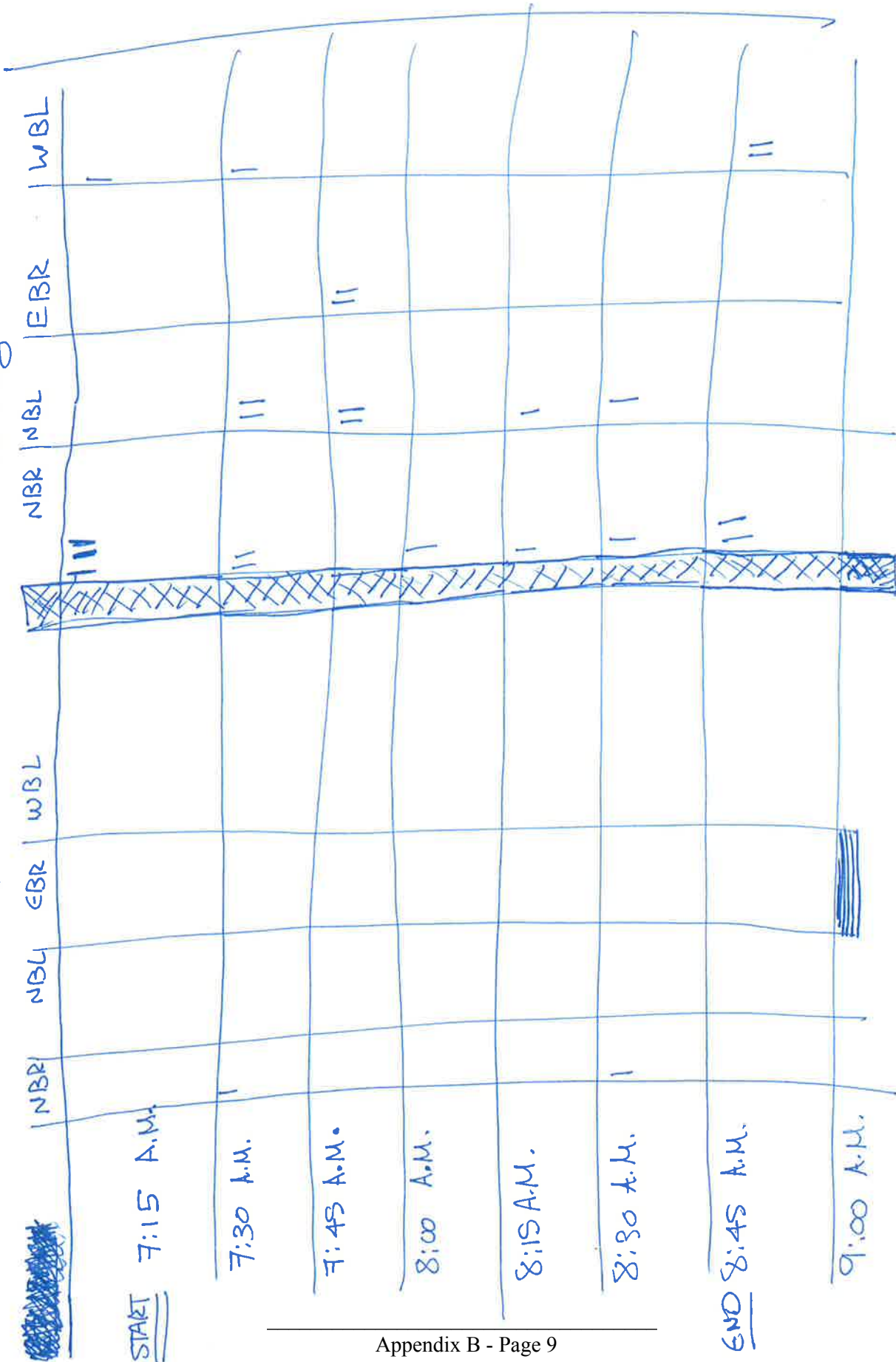
Groups Printed- Unshifted

Start Time	KGB From North					ENTER From East					KGB From South					ENTER From West					Int. Total
	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	Rig ht	Thru	Left	Ped s	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
04:00 PM	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0	1	0	4	0	5	9
04:15 PM	3	0	0	0	3	0	0	0	0	0	0	0	2	0	2	0	0	2	0	2	7
04:30 PM	3	0	0	0	3	0	0	0	0	0	0	0	1	0	1	0	0	6	0	6	10
04:45 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	7
Total	13	0	0	0	13	0	0	0	0	0	0	0	3	0	3	1	0	16	0	17	33
05:00 PM	3	0	0	0	3	0	0	0	0	0	0	0	1	0	1	0	0	4	0	4	8
05:15 PM	7	0	0	0	7	0	0	0	0	0	0	0	1	0	1	1	0	0	0	1	9
05:30 PM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	5
Grand Total	24	0	0	0	24	0	0	0	0	0	0	0	5	0	5	2	0	24	0	26	55
Apprch %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		7.7	0.0	92.3	0.0		
Total %	43.6	0.0	0.0	0.0	43.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0	9.1	0.0	9.1	3.6	0.0	43.6	0.0	47.3	

A.M.
7:00 Missed

* Driveaway 2
Bus @ 7:50 A.M.

Driveaway 1



Time	Drive way 1				* Drive way 2			
	EBR	WBL	NBL	NBR	EBR	NBL	NBR	NBL
4:00								
4:15								
4:30				↓ school bus	10	↓ same bus		
4:45								
5:00								
5:15								
5:30								
5:45								

Wave
Fair

SCF VALLEY MEDICAL COMPLEX PHASE 1 TIA

RAW TRAFFIC DATA

Data Collected By:
 DOWL HKM
 4041 B Street, Anchorage, AK
 (907) 562-2000

File Name : KGBENT~1
 Site Code : 00000000
 Start Date : 12/08/2010
 Page No : 1

Groups Printed- Unshifted

Start Time	KNIK GOOSE BAY ROAD From North					ENTER WAY From East					KNIK GOOSE BAY ROAD From South					ENTER WAY From West					Int. Total	
	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total		
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0			
07:30 AM	5	1	0	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6
Grand Total	5	1	0	0	6	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6
Apprch %	83.3	16.7	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0			
Total %	83.3	16.7	0.0	0.0	100.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	

SCF VALLEY MEDICAL COMPLEX PHASE 1 TIA

RAW TRAFFIC DATA

Data Collected By:
 DOWL HKM
 4041 B Street, Anchorage, AK
 (907) 562-2000

File Name : KGBENT~2
 Site Code : 00000000
 Start Date : 12/07/2010
 Page No : 1

Groups Printed- Unshifted

Start Time	KNIK GOOSE BAY ROAD From North					ENTER WAY From East					KNIK GOOSE BAY ROAD From South					ENTER WAY From West					Int. Total
	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	
Factor	1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0	1.0		
05:45 PM	7	0	0	0	7	0	0	0	0	0	0	0	1	0	1	1	0	4	0	5	13
Total	7	0	0	0	7	0	0	0	0	0	0	0	1	0	1	1	0	4	0	5	13
Grand Total	7	0	0	0	7	0	0	0	0	0	0	0	1	0	1	1	0	4	0	5	13
Apprch %	100.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0		0.0	0.0	100.0	0.0		20.0	0.0	80.0	0.0		
Total %	53.8	0.0	0.0	0.0	53.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	7.7	0.0	7.7	7.7	0.0	30.8	0.0	38.5	

APPENDIX C

Background and Total Traffic Analysis

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1-6-2011	Analysis Year	2010					
Analysis Time Period	2010							
Project Description 2010 A.M. Existing Traffic - Driveway 1/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 1					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	536	0	0	149	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	582	0	0	161	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	1	0	0	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	1	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					1			
C (m) (vph)					461			
v/c					0.00			
95% queue length					0.01			
Control Delay					12.8			
LOS					B			
Approach Delay	--	--	12.8					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/6/2011	Analysis Year	2010					
Analysis Time Period	2010							
Project Description 2010 P.M. Existing Traffic - Driveway 1/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 1					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	471	1	0	917	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	523	1	0	1018	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	2	0	0	0		
Peak-Hour Factor, PHF			0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	2	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					2			
C (m) (vph)					503			
v/c					0.00			
95% queue length					0.01			
Control Delay					12.2			
LOS					B			
Approach Delay	--	--	12.2					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		

Project Description <i>2012 A.M. Background - Driveway 1/Palmer-Wasilla Hwy</i>	
East/West Street: <i>Palmer-Wasilla Highway</i>	North/South Street: <i>Driveway 1</i>
Intersection Orientation: <i>East-West</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	<i>0</i>	<i>585</i>	<i>0</i>	<i>0</i>	<i>163</i>	<i>0</i>
Peak-Hour Factor, PHF	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>
Hourly Flow Rate, HFR	<i>0</i>	<i>635</i>	<i>0</i>	<i>0</i>	<i>177</i>	<i>0</i>
Percent Heavy Vehicles	<i>0</i>	<i>--</i>	<i>--</i>	<i>0</i>	<i>--</i>	<i>--</i>
Median Type	<i>Raised curb</i>					
RT Channelized			<i>0</i>			<i>0</i>
Lanes	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>2</i>	<i>0</i>
Configuration			<i>TR</i>		<i>T</i>	
Upstream Signal		<i>1</i>			<i>0</i>	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>0</i>
Peak-Hour Factor, PHF	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>
Hourly Flow Rate, HFR	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>0</i>
Percent Heavy Vehicles	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>
Percent Grade (%)	<i>2</i>			<i>-2</i>		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		<i>0</i>			<i>0</i>	
RT Channelized			<i>0</i>			<i>0</i>
Lanes	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>0</i>
Configuration			<i>R</i>			

Delay, Queue Length, and Level of Service

Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					<i>R</i>			
v (vph)					<i>1</i>			
C (m) (vph)					<i>426</i>			
v/c					<i>0.00</i>			
95% queue length					<i>0.01</i>			
Control Delay					<i>13.5</i>			
LOS					<i>B</i>			
Approach Delay	<i>--</i>	<i>--</i>	<i>13.5</i>					
Approach LOS	<i>--</i>	<i>--</i>	<i>B</i>					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 A.M. Total Traffic - Driveway 1/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 1					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	681	0	0	201	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	740	0	0	218	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	1	0	0	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	1	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					1			
C (m) (vph)					364			
v/c					0.00			
95% queue length					0.01			
Control Delay					14.9			
LOS					B			
Approach Delay	--	--	14.9					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 P.M. Background - Driveway 1/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 1					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	514	1	0	1001	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	571	1	0	1112	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	2	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	2	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					2			
C (m) (vph)					468			
v/c					0.00			
95% queue length					0.01			
Control Delay					12.7			
LOS					B			
Approach Delay	--	--	12.7					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 P.M. Total Traffic - Driveway 1/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 1					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	582	1	0	1179	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	646	1	0	1310	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	2	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	2	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					2			
C (m) (vph)					419			
v/c					0.00			
95% queue length					0.01			
Control Delay					13.6			
LOS					B			
Approach Delay	--	--	13.6					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Adam McGill	Intersection	
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF
Date Performed	1/12/2011	Analysis Year	2022
Analysis Time Period	2022		

Project Description 2022 A.M. Background - Driveway 1/Palmer-Wasilla Hwy	
East/West Street: Palmer-Wasilla Highway	North/South Street: Driveway 1
Intersection Orientation: East-West	Study Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	0	909	0	0	253	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	988	0	0	274	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Raised curb					
RT Channelized			0			0
Lanes	0	1	0	0	2	0
Configuration			TR		T	
Upstream Signal		1			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	0	2	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	0	2	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	2			-2		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	1	0	0	0
Configuration			R			

Delay, Queue Length, and Level of Service

Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					2			
C (m) (vph)					249			
v/c					0.01			
95% queue length					0.02			
Control Delay					19.6			
LOS					C			
Approach Delay	--	--	19.6					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 A.M. Total Traffic - Driveway 1/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 1					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	1006	0	0	295	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	1093	0	0	320	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	2	0	0	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	2	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					2			
C (m) (vph)					212			
v/c					0.01			
95% queue length					0.03			
Control Delay					22.1			
LOS					C			
Approach Delay	--	--	22.1					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 P.M. Background - Driveway 1/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 1					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	799	2	0	1555	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	887	2	0	1727	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	3	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	3	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					3			
C (m) (vph)					291			
v/c					0.01			
95% queue length					0.03			
Control Delay					17.5			
LOS					C			
Approach Delay	--	--	17.5					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 P.M. Total Traffic - Driveway 1/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 1					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	872	2	0	1739	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	968	2	0	1932	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	3	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	3	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					3			
C (m) (vph)					257			
v/c					0.01			
95% queue length					0.04			
Control Delay					19.2			
LOS					C			
Approach Delay	--	--	19.2					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/6/2011	Analysis Year	2010					
Analysis Time Period	2010							
Project Description 2010 A.M. Existing Traffic - Driveway 2/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 2					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	534	2	2	149	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	580	2	2	161	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR	LT	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	4	0	6	0	0	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	4	0	6	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (vph)		2		10				
C (m) (vph)		1002		460				
v/c		0.00		0.02				
95% queue length		0.01		0.07				
Control Delay		8.6		13.0				
LOS		A		B				
Approach Delay	--	--	13.0					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	<i>Adam McGill</i>	Intersection						
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>					
Date Performed	<i>1/6/2011</i>	Analysis Year	<i>2010</i>					
Analysis Time Period	<i>2010</i>							
Project Description <i>2010 P.M. Existing Traffic - Driveway 2/Palmer-Wasilla Hwy</i>								
East/West Street: <i>Palmer-Wasilla Highway</i>			North/South Street: <i>Driveway 2</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	<i>0</i>	<i>469</i>	<i>4</i>	<i>9</i>	<i>917</i>	<i>0</i>		
Peak-Hour Factor, PHF	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>		
Hourly Flow Rate, HFR	<i>0</i>	<i>521</i>	<i>4</i>	<i>10</i>	<i>1018</i>	<i>0</i>		
Percent Heavy Vehicles	<i>0</i>	<i>--</i>	<i>--</i>	<i>0</i>	<i>--</i>	<i>--</i>		
Median Type	<i>Raised curb</i>							
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>2</i>	<i>0</i>		
Configuration			<i>TR</i>	<i>LT</i>	<i>T</i>			
Upstream Signal		<i>1</i>			<i>0</i>			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	<i>1</i>	<i>0</i>	<i>7</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Peak-Hour Factor, PHF	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>		
Hourly Flow Rate, HFR	<i>1</i>	<i>0</i>	<i>7</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Heavy Vehicles	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Grade (%)	<i>2</i>			<i>-2</i>				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		<i>0</i>			<i>0</i>			
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Configuration		<i>LR</i>						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		<i>LT</i>		<i>LR</i>				
v (vph)		<i>10</i>		<i>8</i>				
C (m) (vph)		<i>1052</i>		<i>480</i>				
v/c		<i>0.01</i>		<i>0.02</i>				
95% queue length		<i>0.03</i>		<i>0.05</i>				
Control Delay		<i>8.5</i>		<i>12.6</i>				
LOS		<i>A</i>		<i>B</i>				
Approach Delay	<i>--</i>	<i>--</i>	<i>12.6</i>					
Approach LOS	<i>--</i>	<i>--</i>	<i>B</i>					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 A.M. Background - Driveway 2/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 2					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	583	2	2	159	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	633	2	2	172	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR	LT	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	4	0	7	0	0	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	4	0	7	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (vph)		2		11				
C (m) (vph)		958		428				
v/c		0.00		0.03				
95% queue length		0.01		0.08				
Control Delay		8.8		13.6				
LOS		A		B				
Approach Delay	--	--	13.6					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 A.M. Total Traffic - Driveway 2/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 2					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	677	4	0	197	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	735	4	0	214	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	11	0	0	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	11	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					11			
C (m) (vph)					365			
v/c					0.03			
95% queue length					0.09			
Control Delay					15.2			
LOS					C			
Approach Delay	--	--	15.2					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	<i>Adam McGill</i>	Intersection						
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>					
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>					
Analysis Time Period	<i>2012</i>							
Project Description <i>2012 P.M. Background - Driveway 2/Palmer-Wasilla Hwy</i>								
East/West Street: <i>Palmer-Wasilla Highway</i>			North/South Street: <i>Driveway 2</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	<i>0</i>	<i>512</i>	<i>4</i>	<i>10</i>	<i>1001</i>	<i>0</i>		
Peak-Hour Factor, PHF	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>		
Hourly Flow Rate, HFR	<i>0</i>	<i>568</i>	<i>4</i>	<i>11</i>	<i>1112</i>	<i>0</i>		
Percent Heavy Vehicles	<i>0</i>	<i>--</i>	<i>--</i>	<i>0</i>	<i>--</i>	<i>--</i>		
Median Type	<i>Raised curb</i>							
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>2</i>	<i>0</i>		
Configuration			<i>TR</i>	<i>LT</i>	<i>T</i>			
Upstream Signal		<i>1</i>			<i>0</i>			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	<i>1</i>	<i>0</i>	<i>8</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Peak-Hour Factor, PHF	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>		
Hourly Flow Rate, HFR	<i>1</i>	<i>0</i>	<i>8</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Heavy Vehicles	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Grade (%)	<i>2</i>			<i>-2</i>				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		<i>0</i>			<i>0</i>			
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Configuration		<i>LR</i>						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		<i>LT</i>		<i>LR</i>				
v (vph)		<i>11</i>		<i>9</i>				
C (m) (vph)		<i>1011</i>		<i>449</i>				
v/c		<i>0.01</i>		<i>0.02</i>				
95% queue length		<i>0.03</i>		<i>0.06</i>				
Control Delay		<i>8.6</i>		<i>13.2</i>				
LOS		<i>A</i>		<i>B</i>				
Approach Delay	<i>--</i>	<i>--</i>	<i>13.2</i>					
Approach LOS	<i>--</i>	<i>--</i>	<i>B</i>					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 P.M. Total Traffic - Driveway 2/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 2					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	570	14	0	1179	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	633	15	0	1310	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	9	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	10	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					10			
C (m) (vph)					423			
v/c					0.02			
95% queue length					0.07			
Control Delay					13.7			
LOS					B			
Approach Delay	--	--	13.7					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 A.M. Background - Driveway 2/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 2					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	908	3	3	253	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	986	3	3	274	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR	LT	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	7	0	10	0	0	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	7	0	10	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (vph)		3		17				
C (m) (vph)		707		260				
v/c		0.00		0.07				
95% queue length		0.01		0.21				
Control Delay		10.1		19.8				
LOS		B		C				
Approach Delay	--	--	19.8					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 A.M. Total Traffic - Driveway 2/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 2					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	1002	6	0	295	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	1089	6	0	320	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR		T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	17	0	0	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	18	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (vph)					18			
C (m) (vph)					213			
v/c					0.08			
95% queue length					0.27			
Control Delay					23.5			
LOS					C			
Approach Delay	--	--	23.5					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 P.M. Background - Driveway 2/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Driveway 2					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	795	7	15	1555	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	883	7	16	1727	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration			TR	LT	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	2	0	12	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	2	0	13	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (vph)		16		15				
C (m) (vph)		770		274				
v/c		0.02		0.05				
95% queue length		0.06		0.17				
Control Delay		9.8		18.9				
LOS		A		C				
Approach Delay	--	--	18.9					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	<i>Adam McGill</i>	Intersection						
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>					
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>					
Analysis Time Period	<i>2022</i>							
Project Description <i>2022 P.M. Total Traffic - Driveway 2/Palmer-Wasilla Hwy</i>								
East/West Street: <i>Palmer-Wasilla Highway</i>			North/South Street: <i>Driveway 2</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	<i>0</i>	<i>853</i>	<i>22</i>	<i>0</i>	<i>1739</i>	<i>0</i>		
Peak-Hour Factor, PHF	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>		
Hourly Flow Rate, HFR	<i>0</i>	<i>947</i>	<i>24</i>	<i>0</i>	<i>1932</i>	<i>0</i>		
Percent Heavy Vehicles	<i>0</i>	<i>--</i>	<i>--</i>	<i>0</i>	<i>--</i>	<i>--</i>		
Median Type	<i>Raised curb</i>							
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>2</i>	<i>0</i>		
Configuration			<i>TR</i>		<i>T</i>			
Upstream Signal		<i>1</i>			<i>0</i>			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	<i>0</i>	<i>0</i>	<i>14</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Peak-Hour Factor, PHF	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>	<i>0.90</i>		
Hourly Flow Rate, HFR	<i>0</i>	<i>0</i>	<i>15</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Heavy Vehicles	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Grade (%)	<i>2</i>			<i>-2</i>				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		<i>0</i>			<i>0</i>			
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Configuration			<i>R</i>					
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					<i>R</i>			
v (vph)					<i>15</i>			
C (m) (vph)					<i>261</i>			
v/c					<i>0.06</i>			
95% queue length					<i>0.18</i>			
Control Delay					<i>19.6</i>			
LOS					<i>C</i>			
Approach Delay	<i>--</i>	<i>--</i>	<i>19.6</i>					
Approach LOS	<i>--</i>	<i>--</i>	<i>C</i>					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/6/2011</i>	Analysis Year	<i>2010</i>
Analysis Time Period	<i>2010</i>		
Project Description <i>2010 A.M. Existing Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	0	455	0	0	211	11
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	0	535	0	0	248	12
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LT</i>					<i>TR</i>
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	0	0	4	0	1
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	0	0	0	4	0	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (vph)	0						5	
C (m) (vph)	1316						417	
v/c	0.00						0.01	
95% queue length	0.00						0.04	
Control Delay	7.7						13.7	
LOS	<i>A</i>						<i>B</i>	
Approach Delay	--	--					13.7	
Approach LOS	--	--					<i>B</i>	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/6/2011</i>	Analysis Year	<i>2010</i>
Analysis Time Period	<i>2010</i>		
Project Description <i>2010 P.M. Existing Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	425	0	0	548	16
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	3	438	0	0	564	16
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LT</i>					<i>TR</i>
Upstream Signal		1			0	

Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	0	0	14	0	1
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	0	0	0	14	0	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (vph)	3						15	
C (m) (vph)	1004						274	
v/c	0.00						0.05	
95% queue length	0.01						0.17	
Control Delay	8.6						18.9	
LOS	<i>A</i>						<i>C</i>	
Approach Delay	--	--					18.9	
Approach LOS	--	--					<i>C</i>	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		
Project Description <i>2012 A.M. Background - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	0	497	0	0	230	12
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	0	584	0	0	270	14
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LT</i>					<i>TR</i>
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	0	0	4	0	1
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	0	0	0	4	0	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (vph)	0						5	
C (m) (vph)	1290						382	
v/c	0.00						0.01	
95% queue length	0.00						0.04	
Control Delay	7.8						14.5	
LOS	<i>A</i>						<i>B</i>	
Approach Delay	--	--					14.5	
Approach LOS	--	--					<i>B</i>	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		
Project Description <i>2012 A.M. Total Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	0	497	133	29	230	12
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	0	584	156	34	270	14
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LTR</i>			<i>LTR</i>		
Upstream Signal		1			0	

Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	7	0	16	4	0	1
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	8	0	18	4	0	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

Delay, Queue Length, and Level of Service

Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LTR</i>	<i>LTR</i>		<i>LTR</i>			<i>LTR</i>	
v (vph)	0	34		26			5	
C (m) (vph)	1290	861		355			241	
v/c	0.00	0.04		0.07			0.02	
95% queue length	0.00	0.12		0.24			0.06	
Control Delay	7.8	9.4		15.9			20.3	
LOS	A	A		C			C	
Approach Delay	--	--	15.9			20.3		
Approach LOS	--	--	C			C		

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		
Project Description <i>2012 P.M. Background - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	464	0	0	588	17
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	3	478	0	0	606	17
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LT</i>					<i>TR</i>
Upstream Signal		1			0	

Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	0	0	15	0	1
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	0	0	0	15	0	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

Delay, Queue Length, and Level of Service

Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (vph)	3						16	
C (m) (vph)	968						242	
v/c	0.00						0.07	
95% queue length	0.01						0.21	
Control Delay	8.7						20.9	
LOS	<i>A</i>						<i>C</i>	
Approach Delay	--	--					20.9	
Approach LOS	--	--					<i>C</i>	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		
Project Description <i>2012 P.M. Total Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	464	27	19	598	17
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	3	478	27	19	616	17
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LTR</i>			<i>LTR</i>		
Upstream Signal		1			0	

Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	45	2	41	15	1	1
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	46	2	42	15	1	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

Delay, Queue Length, and Level of Service

Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LTR</i>	<i>LTR</i>		<i>LTR</i>			<i>LTR</i>	
v (vph)	3	19		90			17	
C (m) (vph)	960	1070		253			159	
v/c	0.00	0.02		0.36			0.11	
95% queue length	0.01	0.05		1.54			0.35	
Control Delay	8.8	8.4		26.9			30.3	
LOS	A	A		D			D	
Approach Delay	--	--	26.9			30.3		
Approach LOS	--	--	D			D		

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		
Project Description <i>2022 A.M. Background - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	<i>0</i>	<i>772</i>	<i>0</i>	<i>0</i>	<i>358</i>	<i>19</i>
Peak-Hour Factor, PHF	<i>0.85</i>	<i>0.85</i>	<i>0.85</i>	<i>0.85</i>	<i>0.85</i>	<i>0.85</i>
Hourly Flow Rate, HFR	<i>0</i>	<i>908</i>	<i>0</i>	<i>0</i>	<i>421</i>	<i>22</i>
Percent Heavy Vehicles	<i>0</i>	<i>--</i>	<i>--</i>	<i>0</i>	<i>--</i>	<i>--</i>
Median Type	<i>Undivided</i>					
RT Channelized			<i>0</i>			<i>0</i>
Lanes	<i>0</i>	<i>2</i>	<i>0</i>	<i>0</i>	<i>2</i>	<i>0</i>
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>
Upstream Signal		<i>1</i>			<i>0</i>	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	<i>0</i>	<i>0</i>	<i>0</i>	<i>7</i>	<i>0</i>	<i>2</i>
Peak-Hour Factor, PHF	<i>0.85</i>	<i>0.85</i>	<i>0.85</i>	<i>0.85</i>	<i>0.85</i>	<i>0.85</i>
Hourly Flow Rate, HFR	<i>0</i>	<i>0</i>	<i>0</i>	<i>8</i>	<i>0</i>	<i>2</i>
Percent Heavy Vehicles	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>
Percent Grade (%)	<i>0</i>			<i>0</i>		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		<i>0</i>			<i>0</i>	
RT Channelized			<i>0</i>			<i>0</i>
Lanes	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>
Configuration					<i>LR</i>	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (vph)	<i>0</i>						<i>10</i>	
C (m) (vph)	<i>1128</i>						<i>390</i>	
v/c	<i>0.00</i>						<i>0.03</i>	
95% queue length	<i>0.00</i>						<i>0.08</i>	
Control Delay	<i>8.2</i>						<i>14.5</i>	
LOS	<i>A</i>						<i>B</i>	
Approach Delay	<i>--</i>	<i>--</i>					<i>14.5</i>	
Approach LOS	<i>--</i>	<i>--</i>					<i>B</i>	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		
Project Description <i>2022 A.M. Total Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	0	772	58	29	358	19
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	0	908	68	34	421	22
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	LT		TR	LT		TR
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	7	0	16	7	0	2
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	8	0	18	8	0	2
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			2		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		LTR			LTR	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT		LTR			LTR	
v (vph)	0	34		26			10	
C (m) (vph)	1128	790		359			310	
v/c	0.00	0.04		0.07			0.03	
95% queue length	0.00	0.13		0.23			0.10	
Control Delay	8.2	9.8		15.8			17.0	
LOS	A	A		C			C	
Approach Delay	--	--		15.8			17.0	
Approach LOS	--	--		C			C	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		
Project Description <i>2022 P.M. Background - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	5	721	0	0	929	27
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	5	743	0	0	957	27
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	LT	T			T	TR
Upstream Signal		1			0	

Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	0	0	24	0	2
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	0	0	0	24	0	2
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (vph)	5						26	
C (m) (vph)	710						172	
v/c	0.01						0.15	
95% queue length	0.02						0.52	
Control Delay	10.1						29.6	
LOS	B						D	
Approach Delay	--	--					29.6	
Approach LOS	--	--					D	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		
Project Description <i>2022 P.M. Total Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	5	721	27	19	929	27
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	5	743	27	19	957	27
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	LT		TR	LT		TR
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	45	2	41	24	1	2
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	46	2	42	24	1	2
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		LTR			LTR	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT		LTR			LTR	
v (vph)	5	19		90			27	
C (m) (vph)	710	921		221			115	
v/c	0.01	0.02		0.41			0.23	
95% queue length	0.02	0.06		1.85			0.85	
Control Delay	10.1	9.0		32.0			45.7	
LOS	B	A		D			E	
Approach Delay	--	--		32.0			45.7	
Approach LOS	--	--		D			E	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/6/2011	Analysis Year	2010					
Analysis Time Period	2010							
Project Description 2010 A.M. Existing Traffic - Glenwood Ave/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Glenwood Avenue					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	516	9	6	134	0		
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Hourly Flow Rate, HFR	0	543	9	6	141	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	1	1	0	1	2	0		
Configuration	L		TR	L	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	13	504	10	0	0	0		
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Hourly Flow Rate, HFR	13	0	10	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L		LR				
v (vph)	0	6		23				
C (m) (vph)	1455	1028		480				
v/c	0.00	0.01		0.05				
95% queue length	0.00	0.02		0.15				
Control Delay	7.5	8.5		12.9				
LOS	A	A		B				
Approach Delay	--	--	12.9					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Adam McGill	Intersection	
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF
Date Performed	1/6/2011	Analysis Year	2010
Analysis Time Period	2010		

Project Description 2010 P.M. Existing Traffic- Glenwood Ave/Palmer-Wasilla Hwy	
East/West Street: Palmer-Wasilla Highway	North/South Street: Glenwood Avenue
Intersection Orientation: East-West	Study Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	0	431	20	9	825	0
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Hourly Flow Rate, HFR	0	478	22	10	916	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Raised curb					
RT Channelized			0			0
Lanes	1	1	0	1	2	0
Configuration	L		TR	L	T	
Upstream Signal		1			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	37	504	13	0	0	0
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Hourly Flow Rate, HFR	41	0	14	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	2			-2		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration		LR				

Delay, Queue Length, and Level of Service

Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L		LR				
v (vph)	0	10		55				
C (m) (vph)	753	1075		413				
v/c	0.00	0.01		0.13				
95% queue length	0.00	0.03		0.46				
Control Delay	9.8	8.4		15.1				
LOS	A	A		C				
Approach Delay	--	--		15.1				
Approach LOS	--	--		C				

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 A.M. Background - Glenwood Ave/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Glenwood Avenue					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	563	10	7	146	0		
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Hourly Flow Rate, HFR	0	592	10	7	153	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	1	1	0	1	2	0		
Configuration	L		TR	L	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	14	0	11	0	0	0		
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Hourly Flow Rate, HFR	14	0	11	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L		LR				
v (vph)	0	7		25				
C (m) (vph)	1440	985		449				
v/c	0.00	0.01		0.06				
95% queue length	0.00	0.02		0.18				
Control Delay	7.5	8.7		13.5				
LOS	A	A		B				
Approach Delay	--	--	13.5					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 A.M. Total Traffic - Glenwood Ave/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Glenwood Avenue					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	4	582	10	7	164	0		
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Hourly Flow Rate, HFR	4	612	10	7	172	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	1	1	0	1	2	0		
Configuration	L		TR	L	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	16	0	11	0	0	0		
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Hourly Flow Rate, HFR	16	0	11	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LR					
v (vph)	4	7	27					
C (m) (vph)	1417	969	433					
v/c	0.00	0.01	0.06					
95% queue length	0.01	0.02	0.20					
Control Delay	7.5	8.7	13.9					
LOS	A	A	B					
Approach Delay	--	--	13.9					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 P.M. Background - Glenwood Ave/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Glenwood Avenue					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	471	22	10	901	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	523	24	11	1001	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	1	1	0	1	2	0		
Configuration	L		TR	L	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	40	0	14	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	44	0	15	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L		LR				
v (vph)	0	11		59				
C (m) (vph)	700	1033		383				
v/c	0.00	0.01		0.15				
95% queue length	0.00	0.03		0.54				
Control Delay	10.1	8.5		16.1				
LOS	B	A		C				
Approach Delay	--	--	16.1					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 P.M. Total Traffic - Glenwood Ave/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Glenwood Avenue					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	1	512	24	10	930	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	1	568	26	11	1033	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	1	1	0	1	2	0		
Configuration	L		TR	L	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	42	0	14	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	46	0	15	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L		LR				
v (vph)	1	11		61				
C (m) (vph)	681	992		360				
v/c	0.00	0.01		0.17				
95% queue length	0.00	0.03		0.60				
Control Delay	10.3	8.7		17.0				
LOS	B	A		C				
Approach Delay	--	--	17.0					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	<i>Adam McGill</i>	Intersection						
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>					
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>					
Analysis Time Period	<i>2022</i>							
Project Description <i>2022 A.M. Background - Glenwood Ave/Palmer-Wasilla Highway</i>								
East/West Street: <i>Palmer-Wasilla Highway</i>			North/South Street: <i>Glenwood Avenue</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	<i>0</i>	<i>875</i>	<i>15</i>	<i>10</i>	<i>227</i>	<i>0</i>		
Peak-Hour Factor, PHF	<i>0.95</i>	<i>0.95</i>	<i>0.95</i>	<i>0.95</i>	<i>0.95</i>	<i>0.95</i>		
Hourly Flow Rate, HFR	<i>0</i>	<i>921</i>	<i>15</i>	<i>10</i>	<i>238</i>	<i>0</i>		
Percent Heavy Vehicles	<i>0</i>	<i>--</i>	<i>--</i>	<i>0</i>	<i>--</i>	<i>--</i>		
Median Type	<i>Raised curb</i>							
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>1</i>	<i>1</i>	<i>0</i>	<i>1</i>	<i>2</i>	<i>0</i>		
Configuration	<i>L</i>		<i>TR</i>	<i>L</i>	<i>T</i>			
Upstream Signal		<i>1</i>			<i>0</i>			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	<i>22</i>	<i>0</i>	<i>17</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Peak-Hour Factor, PHF	<i>0.95</i>	<i>0.95</i>	<i>0.95</i>	<i>0.95</i>	<i>0.95</i>	<i>0.95</i>		
Hourly Flow Rate, HFR	<i>23</i>	<i>0</i>	<i>17</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Heavy Vehicles	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Percent Grade (%)	<i>2</i>			<i>-2</i>				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		<i>0</i>			<i>0</i>			
RT Channelized			<i>0</i>			<i>0</i>		
Lanes	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>		
Configuration		<i>LR</i>						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>L</i>	<i>L</i>		<i>LR</i>				
v (vph)	<i>0</i>	<i>10</i>		<i>40</i>				
C (m) (vph)	<i>1341</i>	<i>740</i>		<i>287</i>				
v/c	<i>0.00</i>	<i>0.01</i>		<i>0.14</i>				
95% queue length	<i>0.00</i>	<i>0.04</i>		<i>0.48</i>				
Control Delay	<i>7.7</i>	<i>9.9</i>		<i>19.6</i>				
LOS	<i>A</i>	<i>A</i>		<i>C</i>				
Approach Delay	<i>--</i>	<i>--</i>	<i>19.6</i>					
Approach LOS	<i>--</i>	<i>--</i>	<i>C</i>					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 A.M. Total Traffic - Glenwood Ave/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Glenwood Avenue					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	7	894	15	10	245	0		
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Hourly Flow Rate, HFR	7	941	15	10	257	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	1	1	0	1	2	0		
Configuration	L		TR	L	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	24	0	17	0	0	0		
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95		
Hourly Flow Rate, HFR	25	0	17	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LR					
v (vph)	7	10	42					
C (m) (vph)	1320	727	277					
v/c	0.01	0.01	0.15					
95% queue length	0.02	0.04	0.53					
Control Delay	7.7	10.0+	20.3					
LOS	A	B	C					
Approach Delay	--	--	20.3					
Approach LOS	--	--	C					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 P.M. Background - Glenwood Ave/Palmer-Wasilla Highway								
East/West Street: Palmer-Wasilla Highway			North/South Street: Glenwood Avenue					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	731	34	15	1399	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	812	37	16	1554	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	1	1	0	1	2	0		
Configuration	L		TR	L	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	63	0	22	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	70	0	24	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LR					
v (vph)	0	16	94					
C (m) (vph)	432	798	240					
v/c	0.00	0.02	0.39					
95% queue length	0.00	0.06	1.76					
Control Delay	13.3	9.6	29.3					
LOS	B	A	D					
Approach Delay	--	--	29.3					
Approach LOS	--	--	D					

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 P.M. Total Traffic - Glenwood Ave/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Glenwood Avenue					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	2	772	36	15	1428	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	2	857	40	16	1586	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Raised curb							
RT Channelized			0			0		
Lanes	1	1	0	1	2	0		
Configuration	L		TR	L	T			
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	65	0	22	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	72	0	24	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L	L	LR					
v (vph)	2	16	96					
C (m) (vph)	420	765	225					
v/c	0.00	0.02	0.43					
95% queue length	0.01	0.06	1.99					
Control Delay	13.6	9.8	32.4					
LOS	B	A	D					
Approach Delay	--	--	32.4					
Approach LOS	--	--	D					

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011


























Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	10	21	128	34	5	458	533	32	180	18
v/c Ratio	0.07	0.11	0.32	0.15	0.01	0.36	0.42	0.05	0.14	0.02
Control Delay	29.4	24.7	27.9	14.5	4.4	8.2	2.3	4.2	6.0	4.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	29.4	24.7	27.9	14.5	4.4	8.2	2.3	4.2	6.0	4.0
Queue Length 50th (ft)	3	5	18	2	1	47	0	2	15	0
Queue Length 95th (ft)	18	27	52	26	5	213	47	15	79	10
Internal Link Dist (ft)		387		251		808			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	136	517	411	551	892	1281	1255	656	1332	1137
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.04	0.31	0.06	0.01	0.36	0.42	0.05	0.14	0.02

Intersection Summary

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				 								
Volume (vph)	9	17	3	118	6	25	5	421	490	29	166	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0
Lane Util. Factor	1.00	1.00		0.97	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.98		1.00	0.88		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	1823		3433	1641		1770	1863	1583	1770	1863	1583
Flt Permitted	0.95	1.00		0.95	1.00		0.64	1.00	1.00	0.44	1.00	1.00
Satd. Flow (perm)	1770	1823		3433	1641		1199	1863	1583	814	1863	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	10	18	3	128	7	27	5	458	533	32	180	18
RTOR Reduction (vph)	0	3	0	0	25	0	0	0	215	0	0	7
Lane Group Flow (vph)	10	18	0	128	9	0	5	458	318	32	180	11
Turn Type	Prot			Prot			pm+pt		Perm	pm+pt		Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases							2		2	6		6
Actuated Green, G (s)	0.8	1.4		5.5	6.1		41.2	40.5	40.5	42.8	41.3	41.3
Effective Green, g (s)	0.8	1.4		5.5	6.1		41.2	40.5	40.5	42.8	41.3	41.3
Actuated g/C Ratio	0.01	0.02		0.08	0.09		0.61	0.60	0.60	0.63	0.61	0.61
Clearance Time (s)	5.0	5.0		5.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	21	38		278	147		733	1111	944	534	1133	963
v/s Ratio Prot	0.01	c0.01		c0.04	c0.01		0.00	c0.25		c0.00	0.10	
v/s Ratio Perm							0.00		0.20	0.04		0.01
v/c Ratio	0.48	0.48		0.46	0.06		0.01	0.41	0.34	0.06	0.16	0.01
Uniform Delay, d1	33.3	32.9		29.8	28.3		5.3	7.3	6.9	4.9	5.8	5.2
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	16.0	9.1		1.2	0.2		0.0	1.1	1.0	0.0	0.3	0.0
Delay (s)	49.4	42.0		31.0	28.5		5.3	8.5	7.9	5.0	6.1	5.3
Level of Service	D	D		C	C		A	A	A	A	A	A
Approach Delay (s)		44.4			30.5			8.1			5.9	
Approach LOS		D			C			A			A	

Intersection Summary

HCM Average Control Delay	11.1	HCM Level of Service	B
HCM Volume to Capacity ratio	0.44		
Actuated Cycle Length (s)	67.9	Sum of lost time (s)	24.0
Intersection Capacity Utilization	48.7%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	25	51	853	144	10	332	399	78	502	16
v/c Ratio	0.17	0.25	0.83	0.22	0.04	0.55	0.51	0.21	0.66	0.02
Control Delay	33.8	24.9	32.5	7.5	13.1	24.4	5.1	14.2	24.4	8.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.8	24.9	32.5	7.5	13.1	24.4	5.1	14.2	24.4	8.9
Queue Length 50th (ft)	10	14	184	9	2	123	0	20	166	0
Queue Length 95th (ft)	33	45	#308	49	11	213	60	46	#403	13
Internal Link Dist (ft)		387		251		808			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	151	476	1029	846	272	608	786	370	760	656
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.11	0.83	0.17	0.04	0.55	0.51	0.21	0.66	0.02
























Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011

													
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations				 									
Volume (vph)	23	32	15	785	33	99	9	305	367	72	462	15	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	
Lane Util. Factor	1.00	1.00		0.97	1.00		1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.95		1.00	0.89		1.00	1.00	0.85	1.00	1.00	0.85	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00	
Satd. Flow (prot)	1770	1775		3433	1653		1770	1863	1583	1770	1863	1583	
Flt Permitted	0.95	1.00		0.95	1.00		0.28	1.00	1.00	0.39	1.00	1.00	
Satd. Flow (perm)	1770	1775		3433	1653		514	1863	1583	725	1863	1583	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	25	35	16	853	36	108	10	332	399	78	502	16	
RTOR Reduction (vph)	0	15	0	0	74	0	0	0	265	0	0	10	
Lane Group Flow (vph)	25	36	0	853	70	0	10	332	134	78	502	6	
Turn Type	Prot			Prot			pm+pt		Perm	pm+pt		Perm	
Protected Phases	7	4		3	8		5	2		1	6		
Permitted Phases							2		2	6		6	
Actuated Green, G (s)	1.9	5.2		19.3	22.6		24.7	24.0	24.0	29.3	26.3	26.3	
Effective Green, g (s)	1.9	5.2		19.3	22.6		24.7	24.0	24.0	29.3	26.3	26.3	
Actuated g/C Ratio	0.03	0.07		0.27	0.32		0.35	0.34	0.34	0.41	0.37	0.37	
Clearance Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	47	129		927	522		190	625	531	341	685	582	
v/s Ratio Prot	0.01	c0.02		c0.25	0.04		0.00	0.18		c0.01	c0.27		
v/s Ratio Perm							0.02		0.08	0.08		0.00	
v/c Ratio	0.53	0.28		0.92	0.13		0.05	0.53	0.25	0.23	0.73	0.01	
Uniform Delay, d1	34.4	31.4		25.4	17.5		16.1	19.2	17.2	13.5	19.6	14.3	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	11.1	1.2		14.0	0.1		0.1	3.2	1.1	0.3	6.8	0.0	
Delay (s)	45.4	32.6		39.4	17.6		16.3	22.4	18.4	13.9	26.4	14.4	
Level of Service	D	C		D	B		B	C	B	B	C	B	
Approach Delay (s)		36.8			36.2			20.2			24.4		
Approach LOS		D			D			C			C		
Intersection Summary													
HCM Average Control Delay			28.4									HCM Level of Service	C
HCM Volume to Capacity ratio			0.69										
Actuated Cycle Length (s)			71.5									Sum of lost time (s)	15.0
Intersection Capacity Utilization			69.2%									ICU Level of Service	C
Analysis Period (min)			15										
c	Critical Lane Group												

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	11	24	140	37	5	500	582	35	197	21
v/c Ratio	0.08	0.12	0.40	0.14	0.01	0.40	0.47	0.06	0.15	0.02
Control Delay	31.2	26.4	31.3	13.8	5.8	10.3	2.6	5.7	7.1	4.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.2	26.4	31.3	13.8	5.8	10.3	2.6	5.7	7.1	4.4
Queue Length 50th (ft)	3	6	19	2	1	47	0	3	15	0
Queue Length 95th (ft)	20	29	57	28	5	243	50	16	88	11
Internal Link Dist (ft)		387		251		808			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	135	514	350	521	821	1240	1248	567	1291	1103
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.05	0.40	0.07	0.01	0.40	0.47	0.06	0.15	0.02

Intersection Summary

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	10	19	3	129	7	27	5	460	535	32	181	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0		5.5	5.0	5.0	5.5	5.0	5.0
Lane Util. Factor	1.00	1.00		0.97	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.98		1.00	0.88		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	1828		3433	1644		1770	1863	1583	1770	1863	1583
Flt Permitted	0.95	1.00		0.95	1.00		0.63	1.00	1.00	0.39	1.00	1.00
Satd. Flow (perm)	1770	1828		3433	1644		1181	1863	1583	734	1863	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	11	21	3	140	8	29	5	500	582	35	197	21
RTOR Reduction (vph)	0	3	0	0	26	0	0	0	250	0	0	9
Lane Group Flow (vph)	11	21	0	140	11	0	5	500	332	35	197	12
Turn Type	Prot			Prot			pm+pt		Perm	pm+pt		Perm
Protected Phases	7	4		3	8		5	2		1		6
Permitted Phases							2		2	6		6
Actuated Green, G (s)	0.8	3.4		4.8	7.4		40.6	39.9	39.9	42.0	40.6	40.6
Effective Green, g (s)	0.8	3.4		4.8	7.4		40.6	39.9	39.9	42.0	40.6	40.6
Actuated g/C Ratio	0.01	0.05		0.07	0.11		0.58	0.57	0.57	0.60	0.58	0.58
Clearance Time (s)	5.0	5.0		5.0	5.0		5.5	5.0	5.0	5.5	5.0	5.0
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	20	89		235	174		691	1062	902	461	1081	918
v/s Ratio Prot	0.01	c0.01		c0.04	c0.01		0.00	c0.27		c0.00	0.11	
v/s Ratio Perm							0.00		0.21	0.04		0.01
v/c Ratio	0.55	0.24		0.60	0.06		0.01	0.47	0.37	0.08	0.18	0.01
Uniform Delay, d1	34.4	32.1		31.7	28.2		6.2	8.8	8.2	6.1	6.9	6.2
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	28.9	1.4		4.0	0.2		0.0	1.5	1.2	0.1	0.4	0.0
Delay (s)	63.3	33.4		35.7	28.3		6.2	10.3	9.3	6.2	7.3	6.2
Level of Service	E	C		D	C		A	B	A	A	A	A
Approach Delay (s)		42.8			34.1			9.8			7.0	
Approach LOS		D			C			A			A	

Intersection Summary

HCM Average Control Delay	12.9	HCM Level of Service	B
HCM Volume to Capacity ratio	0.49		
Actuated Cycle Length (s)	70.0	Sum of lost time (s)	25.5
Intersection Capacity Utilization	52.7%	ICU Level of Service	A
Analysis Period (min)	15		
c Critical Lane Group			

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	15	24	173	37	5	640	582	35	204	21
v/c Ratio	0.11	0.12	0.51	0.14	0.01	0.56	0.49	0.08	0.17	0.02
Control Delay	31.8	26.5	33.4	13.9	5.6	12.8	2.8	5.6	7.2	4.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.8	26.5	33.4	13.9	5.6	12.8	2.8	5.6	7.2	4.4
Queue Length 50th (ft)	4	5	24	2	1	68	0	2	16	0
Queue Length 95th (ft)	24	29	#69	28	5	345	50	16	91	11
Internal Link Dist (ft)		387		251		808			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	132	501	341	510	792	1140	1194	436	1190	1019
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.11	0.05	0.51	0.07	0.01	0.56	0.49	0.08	0.17	0.02

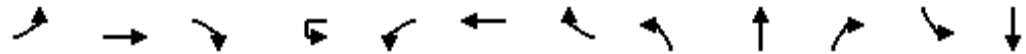
Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Volume (vph)	14	19	3	21	138	7	27	5	589	535	32	188
Ideal Flow (vphp)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00			0.97	1.00		1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.98			1.00	0.88		1.00	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00			0.95	1.00		0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1828			3433	1644		1770	1863	1583	1770	1863
Flt Permitted	0.95	1.00			0.95	1.00		0.63	1.00	1.00	0.28	1.00
Satd. Flow (perm)	1770	1828			3433	1644		1173	1863	1583	527	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	15	21	3	23	150	8	29	5	640	582	35	204
RTOR Reduction (vph)	0	3	0	0	0	25	0	0	0	260	0	0
Lane Group Flow (vph)	15	21	0	0	173	12	0	5	640	322	35	204
Turn Type	Prot			Prot	Prot			pm+pt		Perm	pm+pt	
Protected Phases	7	4		3	3	8		5	2			1
Permitted Phases								2		2		6
Actuated Green, G (s)	0.8	3.5			6.1	8.8		39.5	38.7	38.7	41.1	39.5
Effective Green, g (s)	0.8	3.5			6.1	8.8		39.5	38.7	38.7	41.1	39.5
Actuated g/C Ratio	0.01	0.05			0.09	0.13		0.57	0.55	0.55	0.59	0.57
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	20	92			300	207		670	1031	876	338	1053
v/s Ratio Prot	0.01	c0.01			c0.05	0.01		0.00	c0.34		c0.00	0.11
v/s Ratio Perm								0.00		0.20	0.06	
v/c Ratio	0.75	0.23			0.58	0.06		0.01	0.62	0.37	0.10	0.19
Uniform Delay, d1	34.5	31.9			30.7	26.9		6.6	10.6	8.7	7.3	7.4
Progression Factor	1.00	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	91.0	1.3			2.7	0.1		0.0	2.8	1.2	0.1	0.4
Delay (s)	125.5	33.2			33.3	27.0		6.6	13.4	9.9	7.5	7.8
Level of Service	F	C			C	C		A	B	A	A	A
Approach Delay (s)		68.7				32.2			11.7			7.7
Approach LOS		E				C			B			A

Intersection Summary

HCM Average Control Delay	14.9	HCM Level of Service	B
HCM Volume to Capacity ratio	0.57		
Actuated Cycle Length (s)	69.9	Sum of lost time (s)	20.0
Intersection Capacity Utilization	60.2%	ICU Level of Service	B
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011

Movement	SBR
Lane Configurations	7
Volume (vph)	19
Ideal Flow (vphpl)	1900
Total Lost time (s)	5.0
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.92
Adj. Flow (vph)	21
RTOR Reduction (vph)	9
Lane Group Flow (vph)	12
Turn Type	Perm
Protected Phases	
Permitted Phases	6
Actuated Green, G (s)	39.5
Effective Green, g (s)	39.5
Actuated g/C Ratio	0.57
Clearance Time (s)	5.0
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	895
v/s Ratio Prot	
v/s Ratio Perm	0.01
v/c Ratio	0.01
Uniform Delay, d1	6.7
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	6.7
Level of Service	A
Approach Delay (s)	
Approach LOS	
Intersection Summary	

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	17	65	932	156	11	362	436	86	549	17
v/c Ratio	0.18	0.36	0.87	0.22	0.05	0.52	0.50	0.24	0.68	0.02
Control Delay	47.1	31.6	39.5	7.4	15.7	26.9	4.7	17.8	27.7	9.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	47.1	31.6	39.5	7.4	15.7	26.9	4.7	17.8	27.7	9.1
Queue Length 50th (ft)	10	21	260	12	3	167	0	28	240	0
Queue Length 95th (ft)	32	60	#392	56	14	270	64	59	#499	15
Internal Link Dist (ft)		387		251		808			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	95	366	1151	854	243	698	866	358	809	697
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.18	0.81	0.18	0.05	0.52	0.50	0.24	0.68	0.02
























Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				 								
Volume (vph)	16	35	25	857	36	108	10	333	401	79	505	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		0.97	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.94		1.00	0.89		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	1747		3433	1653		1770	1863	1583	1770	1863	1583
Flt Permitted	0.95	1.00		0.95	1.00		0.23	1.00	1.00	0.37	1.00	1.00
Satd. Flow (perm)	1770	1747		3433	1653		437	1863	1583	692	1863	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	17	38	27	932	39	117	11	362	436	86	549	17
RTOR Reduction (vph)	0	25	0	0	75	0	0	0	272	0	0	10
Lane Group Flow (vph)	17	40	0	932	81	0	11	362	164	86	549	7
Turn Type	Prot			Prot			pm+pt		Perm	pm+pt		Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases							2		2	6		6
Actuated Green, G (s)	1.7	8.1		26.8	33.2		35.7	35.0	35.0	40.3	37.3	37.3
Effective Green, g (s)	1.7	8.1		26.8	33.2		35.7	35.0	35.0	40.3	37.3	37.3
Actuated g/C Ratio	0.02	0.09		0.29	0.36		0.38	0.38	0.38	0.43	0.40	0.40
Clearance Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	32	152		990	591		178	702	596	335	748	636
v/s Ratio Prot	0.01	c0.02		c0.27	0.05		0.00	0.19		c0.01	c0.29	
v/s Ratio Perm							0.02		0.10	0.10		0.00
v/c Ratio	0.53	0.27		0.94	0.14		0.06	0.52	0.28	0.26	0.73	0.01
Uniform Delay, d1	45.2	39.6		32.3	20.2		19.3	22.4	20.1	16.5	23.6	16.7
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	15.9	0.9		16.3	0.1		0.1	2.7	1.1	0.4	6.3	0.0
Delay (s)	61.1	40.6		48.6	20.3		19.5	25.1	21.3	16.9	29.9	16.7
Level of Service	E	D		D	C		B	C	C	B	C	B
Approach Delay (s)		44.8			44.5			23.0			27.8	
Approach LOS		D			D			C			C	

Intersection Summary

HCM Average Control Delay	33.8	HCM Level of Service	C
HCM Volume to Capacity ratio	0.71		
Actuated Cycle Length (s)	92.9	Sum of lost time (s)	15.0
Intersection Capacity Utilization	73.5%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	30	55	1071	158	11	388	436	86	596	20
v/c Ratio	0.22	0.33	0.81	0.21	0.07	0.64	0.54	0.31	0.84	0.02
Control Delay	49.8	40.4	32.2	7.0	23.7	37.8	6.0	26.4	42.1	8.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.8	40.4	32.2	7.0	23.7	37.8	6.0	26.4	42.1	8.9
Queue Length 50th (ft)	18	23	307	16	4	213	0	33	323	1
Queue Length 95th (ft)	52	68	406	56	19	#423	83	82	#769	17
Internal Link Dist (ft)		387		251		808			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	296	340	1915	1001	159	603	807	277	712	965
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.16	0.56	0.16	0.07	0.64	0.54	0.31	0.84	0.02

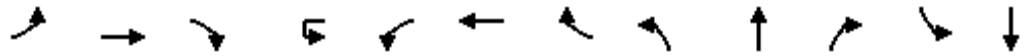
Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Volume (vph)	28	35	16	53	932	38	108	10	357	401	79	548
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00			0.97	1.00		1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.95			1.00	0.89		1.00	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00			0.95	1.00		0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1776			3433	1656		1770	1863	1583	1770	1863
Flt Permitted	0.95	1.00			0.95	1.00		0.12	1.00	1.00	0.30	1.00
Satd. Flow (perm)	1770	1776			3433	1656		227	1863	1583	553	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	30	38	17	58	1013	41	117	11	388	436	86	596
RTOR Reduction (vph)	0	15	0	0	0	72	0	0	0	291	0	0
Lane Group Flow (vph)	30	40	0	0	1071	86	0	11	388	145	86	596
Turn Type	Prot			Prot	Prot			pm+pt		Perm	pm+pt	
Protected Phases	7	4		3	3	8		5	2			6
Permitted Phases								2		2		6
Actuated Green, G (s)	4.4	7.2			35.9	38.7		33.9	33.2	33.2	39.1	35.8
Effective Green, g (s)	4.4	7.2			35.9	38.7		33.9	33.2	33.2	39.1	35.8
Actuated g/C Ratio	0.04	0.07			0.36	0.39		0.34	0.33	0.33	0.39	0.36
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	78	128			1237	643		88	621	528	257	670
v/s Ratio Prot	0.02	c0.02			c0.31	0.05		0.00	0.21		c0.01	c0.32
v/s Ratio Perm								0.04		0.09	0.12	
v/c Ratio	0.38	0.31			0.87	0.13		0.12	0.62	0.28	0.33	0.89
Uniform Delay, d1	46.3	43.9			29.6	19.6		24.7	28.0	24.4	20.7	30.0
Progression Factor	1.00	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	3.1	1.4			6.6	0.1		0.6	4.7	1.3	0.8	16.3
Delay (s)	49.4	45.3			36.2	19.7		25.4	32.7	25.7	21.5	46.3
Level of Service	D	D			D	B		C	C	C	C	D
Approach Delay (s)		46.7				34.1			28.9			42.5
Approach LOS		D				C			C			D

Intersection Summary

HCM Average Control Delay	35.0	HCM Level of Service	D
HCM Volume to Capacity ratio	0.83		
Actuated Cycle Length (s)	99.6	Sum of lost time (s)	20.0
Intersection Capacity Utilization	79.4%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011

Movement	SBR
Lane Configurations	7
Volume (vph)	18
Ideal Flow (vphpl)	1900
Total Lost time (s)	5.0
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.92
Adj. Flow (vph)	20
RTOR Reduction (vph)	11
Lane Group Flow (vph)	9
Turn Type	pm+ov
Protected Phases	7
Permitted Phases	6
Actuated Green, G (s)	40.2
Effective Green, g (s)	40.2
Actuated g/C Ratio	0.40
Clearance Time (s)	5.0
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	718
v/s Ratio Prot	0.00
v/s Ratio Perm	0.01
v/c Ratio	0.01
Uniform Delay, d1	17.8
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	17.8
Level of Service	B
Approach Delay (s)	
Approach LOS	
Intersection Summary	

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	16	37	217	57	9	776	903	53	307	32
v/c Ratio	0.12	0.19	0.53	0.18	0.01	0.39	0.70	0.12	0.14	0.03
Control Delay	33.1	27.2	32.7	12.0	6.2	10.8	4.7	6.4	7.2	4.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	33.1	27.2	32.7	12.0	6.2	10.8	4.7	6.4	7.2	4.2
Queue Length 50th (ft)	5	10	37	4	1	75	0	4	14	0
Queue Length 95th (ft)	25	39	81	34	7	173	63	23	66	14
Internal Link Dist (ft)		387		251		589			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	129	491	411	547	685	1984	1284	437	2194	994
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.12	0.08	0.53	0.10	0.01	0.39	0.70	0.12	0.14	0.03

Intersection Summary

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	15	29	5	200	10	42	8	714	831	49	282	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		0.97	1.00		1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	0.98		1.00	0.88		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	1825		3433	1637		1770	3539	1583	1770	3539	1583
Flt Permitted	0.95	1.00		0.95	1.00		0.56	1.00	1.00	0.30	1.00	1.00
Satd. Flow (perm)	1770	1825		3433	1637		1052	3539	1583	552	3539	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	16	32	5	217	11	46	9	776	903	53	307	32
RTOR Reduction (vph)	0	5	0	0	39	0	0	0	430	0	0	14
Lane Group Flow (vph)	16	32	0	217	18	0	9	776	473	53	307	18
Turn Type	Prot			Prot			pm+pt		Perm	pm+pt		Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases							2		2	6		6
Actuated Green, G (s)	0.8	3.7		7.5	10.4		38.0	37.2	37.2	41.6	39.0	39.0
Effective Green, g (s)	0.8	3.7		7.5	10.4		38.0	37.2	37.2	41.6	39.0	39.0
Actuated g/C Ratio	0.01	0.05		0.11	0.15		0.54	0.52	0.52	0.59	0.55	0.55
Clearance Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	20	95		363	240		571	1854	829	368	1944	870
v/s Ratio Prot	0.01	c0.02		c0.06	0.01		0.00	0.22		c0.01	0.09	
v/s Ratio Perm							0.01		c0.30	0.08		0.01
v/c Ratio	0.80	0.34		0.60	0.07		0.02	0.42	0.57	0.14	0.16	0.02
Uniform Delay, d1	35.0	32.5		30.3	26.1		7.7	10.3	11.5	6.6	7.9	7.3
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	110.1	2.1		2.6	0.1		0.0	0.7	2.8	0.2	0.2	0.0
Delay (s)	145.1	34.6		33.0	26.3		7.7	11.0	14.3	6.8	8.1	7.3
Level of Service	F	C		C	C		A	B	B	A	A	A
Approach Delay (s)		68.0			31.6			12.8			7.8	
Approach LOS		E			C			B			A	

Intersection Summary

HCM Average Control Delay	15.3	HCM Level of Service	B
HCM Volume to Capacity ratio	0.54		
Actuated Cycle Length (s)	71.0	Sum of lost time (s)	20.0
Intersection Capacity Utilization	70.6%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



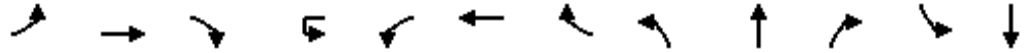
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	21	37	254	57	9	916	903	53	314	32
v/c Ratio	0.16	0.19	0.48	0.21	0.01	0.49	0.73	0.15	0.15	0.03
Control Delay	34.5	27.8	29.6	13.2	7.0	13.0	6.2	7.6	8.1	2.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.5	27.8	29.6	13.2	7.0	13.0	6.2	7.6	8.1	2.6
Queue Length 50th (ft)	8	12	43	4	1	130	12	7	24	0
Queue Length 95th (ft)	30	39	92	33	7	218	121	24	70	11
Internal Link Dist (ft)		387		251		589			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	128	489	539	570	648	1877	1237	356	2075	1179
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.08	0.47	0.10	0.01	0.49	0.73	0.15	0.15	0.03

Intersection Summary

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗			↖	↗		↖	↑↑	↗	↖	↑↑
Volume (vph)	19	29	5	22	212	10	42	8	843	831	49	289
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00			0.97	1.00		1.00	0.95	1.00	1.00	0.95
Frt	1.00	0.98			1.00	0.88		1.00	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00			0.95	1.00		0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1825			3433	1637		1770	3539	1583	1770	3539
Flt Permitted	0.95	1.00			0.95	1.00		0.56	1.00	1.00	0.23	1.00
Satd. Flow (perm)	1770	1825			3433	1637		1045	3539	1583	432	3539
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	21	32	5	24	230	11	46	9	916	903	53	314
RTOR Reduction (vph)	0	5	0	0	0	39	0	0	0	421	0	0
Lane Group Flow (vph)	21	32	0	0	254	18	0	9	916	482	53	314
Turn Type	Prot			Prot	Prot			pm+pt		Perm	pm+pt	
Protected Phases	7	4		3	3	8		5	2			6
Permitted Phases								2		2		6
Actuated Green, G (s)	2.5	2.8			9.7	10.0		36.2	35.4	35.4	39.6	37.1
Effective Green, g (s)	2.5	2.8			9.7	10.0		36.2	35.4	35.4	39.6	37.1
Actuated g/C Ratio	0.04	0.04			0.14	0.14		0.51	0.50	0.50	0.56	0.53
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	63	73			473	233		546	1780	796	291	1865
v/s Ratio Prot	0.01	c0.02			c0.07	0.01		0.00	0.26		c0.01	0.09
v/s Ratio Perm								0.01		c0.30	0.10	
v/c Ratio	0.33	0.44			0.54	0.08		0.02	0.51	0.61	0.18	0.17
Uniform Delay, d1	33.1	33.0			28.3	26.2		8.3	11.7	12.5	7.7	8.6
Progression Factor	1.00	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	3.1	4.2			1.2	0.1		0.0	1.1	3.4	0.3	0.2
Delay (s)	36.2	37.2			29.4	26.3		8.4	12.8	15.9	8.0	8.8
Level of Service	D	D			C	C		A	B	B	A	A
Approach Delay (s)		36.9				28.9			14.3			8.6
Approach LOS		D				C			B			A

Intersection Summary

HCM Average Control Delay	15.7	HCM Level of Service	B
HCM Volume to Capacity ratio	0.56		
Actuated Cycle Length (s)	70.4	Sum of lost time (s)	20.0
Intersection Capacity Utilization	80.6%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011

Movement	SBR
Lane Configurations	
Volume (vph)	29
Ideal Flow (vphpl)	1900
Total Lost time (s)	5.0
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.92
Adj. Flow (vph)	32
RTOR Reduction (vph)	14
Lane Group Flow (vph)	18
Turn Type	pm+ov
Protected Phases	7
Permitted Phases	6
Actuated Green, G (s)	39.6
Effective Green, g (s)	39.6
Actuated g/C Ratio	0.56
Clearance Time (s)	5.0
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	1003
v/s Ratio Prot	0.00
v/s Ratio Perm	0.01
v/c Ratio	0.02
Uniform Delay, d1	6.8
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	6.8
Level of Service	A
Approach Delay (s)	
Approach LOS	
Intersection Summary	

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	42	86	1447	244	16	562	676	133	851	27
v/c Ratio	0.39	0.50	0.93	0.27	0.12	0.62	0.74	0.62	0.76	0.05
Control Delay	63.3	50.4	42.3	6.0	28.2	41.0	8.6	43.6	41.8	13.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	63.3	50.4	42.3	6.0	28.2	41.0	8.6	43.6	41.8	13.6
Queue Length 50th (ft)	30	49	513	26	8	192	0	69	281	1
Queue Length 95th (ft)	70	101	#721	72	25	265	117	#143	#473	25
Internal Link Dist (ft)		387		251		589			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	114	278	1549	979	137	907	909	215	1114	515
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.37	0.31	0.93	0.25	0.12	0.62	0.74	0.62	0.76	0.05

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	39	54	25	1331	56	168	15	517	622	122	783	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00		0.97	1.00		1.00	0.95	1.00	1.00	0.95	1.00
Frt	1.00	0.95		1.00	0.89		1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	1775		3433	1653		1770	3539	1583	1770	3539	1583
Flt Permitted	0.95	1.00		0.95	1.00		0.14	1.00	1.00	0.25	1.00	1.00
Satd. Flow (perm)	1770	1775		3433	1653		255	3539	1583	468	3539	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	42	59	27	1447	61	183	16	562	676	133	851	27
RTOR Reduction (vph)	0	15	0	0	93	0	0	0	492	0	0	17
Lane Group Flow (vph)	42	71	0	1447	151	0	16	562	184	133	851	10
Turn Type	Prot			Prot			pm+pt		Perm	pm+pt		Perm
Protected Phases	7	4		3	8		5	2		2	6	6
Permitted Phases							2		2	6		6
Actuated Green, G (s)	4.1	9.5		50.2	55.6		33.3	31.6	31.6	40.1	35.0	35.0
Effective Green, g (s)	4.1	9.5		50.2	55.6		33.3	31.6	31.6	40.1	35.0	35.0
Actuated g/C Ratio	0.04	0.08		0.43	0.48		0.29	0.27	0.27	0.34	0.30	0.30
Clearance Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	62	145		1481	790		95	961	430	218	1064	476
v/s Ratio Prot	0.02	c0.04		c0.42	0.09		0.00	0.16		c0.03	c0.24	
v/s Ratio Perm							0.05		0.12	0.18		0.01
v/c Ratio	0.68	0.49		0.98	0.19		0.17	0.58	0.43	0.61	0.80	0.02
Uniform Delay, d1	55.5	51.1		32.5	17.5		31.5	36.7	34.9	30.4	37.5	28.6
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	25.5	2.6		18.0	0.1		0.8	2.6	3.1	5.0	6.3	0.1
Delay (s)	81.0	53.8		50.5	17.6		32.3	39.3	38.0	35.4	43.8	28.7
Level of Service	F	D		D	B		C	D	D	D	D	C
Approach Delay (s)		62.7			45.8			38.5			42.3	
Approach LOS		E			D			D			D	

Intersection Summary

HCM Average Control Delay	43.2	HCM Level of Service	D
HCM Volume to Capacity ratio	0.87		
Actuated Cycle Length (s)	116.4	Sum of lost time (s)	20.0
Intersection Capacity Utilization	82.1%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	46	86	1592	246	16	588	676	133	898	29
v/c Ratio	0.45	0.56	1.01	0.28	0.12	0.68	0.75	0.58	0.78	0.04
Control Delay	77.2	65.7	64.0	8.7	33.7	53.1	9.3	44.7	49.2	11.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.2	65.7	64.0	8.7	33.7	53.1	9.3	44.7	49.2	11.3
Queue Length 50th (ft)	41	65	~764	48	9	263	0	84	370	2
Queue Length 95th (ft)	85	119	#923	96	28	332	128	#158	#581	25
Internal Link Dist (ft)		387		563		589			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	116	214	1569	921	129	868	898	228	1151	690
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.40	0.40	1.01	0.27	0.12	0.68	0.75	0.58	0.78	0.04

Intersection Summary

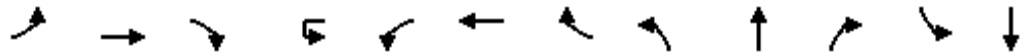
~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations												
Volume (vph)	42	54	25	58	1407	58	168	15	541	622	122	826
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00			0.97	1.00		1.00	0.95	1.00	1.00	0.95
Frt	1.00	0.95			1.00	0.89		1.00	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00			0.95	1.00		0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1775			3433	1655		1770	3539	1583	1770	3539
Flt Permitted	0.95	1.00			0.95	1.00		0.12	1.00	1.00	0.18	1.00
Satd. Flow (perm)	1770	1775			3433	1655		223	3539	1583	339	3539
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	46	59	27	63	1529	63	183	16	588	676	133	898
RTOR Reduction (vph)	0	12	0	0	0	76	0	0	0	515	0	0
Lane Group Flow (vph)	46	74	0	0	1592	170	0	16	588	161	133	898
Turn Type	Prot			Prot	Prot			pm+pt		Prot	pm+pt	
Protected Phases	7	4		3	3	8		5	2	2		6
Permitted Phases								2				6
Actuated Green, G (s)	7.0	12.1			64.0	69.1		35.7	33.4	33.4	48.9	41.6
Effective Green, g (s)	7.0	12.1			64.0	69.1		35.7	33.4	33.4	48.9	41.6
Actuated g/C Ratio	0.05	0.09			0.46	0.49		0.26	0.24	0.24	0.35	0.30
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	89	153			1569	817		82	844	378	226	1052
v/s Ratio Prot	0.03	c0.04			c0.46	0.10		0.00	0.17	0.10	c0.04	c0.25
v/s Ratio Perm								0.05			0.16	
v/c Ratio	0.52	0.48			1.01	0.21		0.20	0.70	0.43	0.59	0.85
Uniform Delay, d1	64.9	61.0			38.0	20.0		41.0	48.7	45.2	34.1	46.3
Progression Factor	1.00	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	5.0	2.4			26.4	0.1		1.2	4.7	3.5	3.9	8.8
Delay (s)	69.8	63.4			64.4	20.1		42.1	53.4	48.7	38.0	55.1
Level of Service	E	E			E	C		D	D	D	D	E
Approach Delay (s)		65.6				58.5			50.8			52.3
Approach LOS		E				E			D			D

Intersection Summary

HCM Average Control Delay	54.9	HCM Level of Service	D
HCM Volume to Capacity ratio	0.91		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	20.0
Intersection Capacity Utilization	106.2%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/20/2011



Movement	SBR
Lane Configurations	
Volume (vph)	27
Ideal Flow (vphpl)	1900
Total Lost time (s)	5.0
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.92
Adj. Flow (vph)	29
RTOR Reduction (vph)	17
Lane Group Flow (vph)	12
Turn Type	pm+ov
Protected Phases	7
Permitted Phases	6
Actuated Green, G (s)	48.6
Effective Green, g (s)	48.6
Actuated g/C Ratio	0.35
Clearance Time (s)	5.0
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	606
v/s Ratio Prot	0.00
v/s Ratio Perm	0.01
v/c Ratio	0.02
Uniform Delay, d1	30.0
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	30.1
Level of Service	C
Approach Delay (s)	
Approach LOS	
Intersection Summary	

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 A.M. Background - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	590	0	0	161	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	641	0	0	174	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration		T			T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	0		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	0	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration								
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								
v (vph)								
C (m) (vph)								
v/c								
95% queue length								
Control Delay								
LOS								
Approach Delay	--	--						
Approach LOS	--	--						

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 A.M. Total Traffic - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	613	0	0	165	20		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	666	0	0	179	21		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration		T			T	TR		
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	24		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	0	0	0	26		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								R
v (vph)								26
C (m) (vph)								943
v/c								0.03
95% queue length								0.08
Control Delay								8.9
LOS								A
Approach Delay	--	--				8.9		
Approach LOS	--	--				A		

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 P.M. Background - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	520	0	0	1011	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	577	0	0	1123	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration		T			T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	0	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration								
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								
v (vph)								
C (m) (vph)								
v/c								
95% queue length								
Control Delay								
LOS								
Approach Delay	--	--						
Approach LOS	--	--						

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 P.M. Total Traffic - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	564	0	0	1012	30		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	626	0	0	1124	33		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration		T			T	TR		
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	119		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	0	0	0	132		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								R
v (vph)								132
C (m) (vph)								464
v/c								0.28
95% queue length								1.16
Control Delay								15.8
LOS								C
Approach Delay	--	--				15.8		
Approach LOS	--	--				C		

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		

Project Description <i>2022 A.M. Background - Site Access/Palmer-Wasilla Hwy</i>	
East/West Street: <i>Palmer-Wasilla Highway</i>	North/South Street: <i>Site Access</i>
Intersection Orientation: <i>East-West</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	<i>0</i>	<i>1440</i>	<i>0</i>	<i>0</i>	<i>401</i>	<i>0</i>
Peak-Hour Factor, PHF	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>
Hourly Flow Rate, HFR	<i>0</i>	<i>1565</i>	<i>0</i>	<i>0</i>	<i>435</i>	<i>0</i>
Percent Heavy Vehicles	<i>0</i>	<i>--</i>	<i>--</i>	<i>0</i>	<i>--</i>	<i>--</i>
Median Type	<i>Undivided</i>					
RT Channelized			<i>0</i>			<i>0</i>
Lanes	<i>0</i>	<i>1</i>	<i>0</i>	<i>0</i>	<i>2</i>	<i>0</i>
Configuration		<i>T</i>			<i>T</i>	
Upstream Signal		<i>0</i>			<i>0</i>	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>
Peak-Hour Factor, PHF	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>	<i>0.92</i>
Hourly Flow Rate, HFR	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>
Percent Heavy Vehicles	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>
Percent Grade (%)	<i>2</i>			<i>-2</i>		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		<i>0</i>			<i>0</i>	
RT Channelized			<i>0</i>			<i>0</i>
Lanes	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>	<i>0</i>
Configuration						

Delay, Queue Length, and Level of Service

Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								
v (vph)								
C (m) (vph)								
v/c								
95% queue length								
Control Delay								
LOS								
Approach Delay	<i>--</i>	<i>--</i>						
Approach LOS	<i>--</i>	<i>--</i>						

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/20/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 A.M. Total Traffic - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	944	0	0	263	20		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	1026	0	0	285	21		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration		T			T	TR		
Upstream Signal		1			1			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	24		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	0	0	0	26		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								R
v (vph)								26
C (m) (vph)								872
v/c								0.03
95% queue length								0.09
Control Delay								9.3
LOS								A
Approach Delay	--	--				9.3		
Approach LOS	--	--				A		

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 P.M. Background - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	807	0	0	1570	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	896	0	0	1744	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration		T			T			
Upstream Signal		0			1			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	0	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration								
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								
v (vph)								
C (m) (vph)								
v/c								
95% queue length								
Control Delay								
LOS								
Approach Delay	--	--						
Approach LOS	--	--						

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/20/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 P.M. Total Traffic - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	852	0	0	1572	30		
Peak-Hour Factor, PHF	0.90	0.90			0.90	0.90		
Hourly Flow Rate, HFR	0	946	0	0	1746	33		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration		T			T	TR		
Upstream Signal		1			1			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	119		
Peak-Hour Factor, PHF						0.90		
Hourly Flow Rate, HFR	0	0	0	0	0	132		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								R
v (vph)								132
C (m) (vph)								638
v/c								0.21
95% queue length								0.77
Control Delay								12.1
LOS								B
Approach Delay	--	--				12.1		
Approach LOS	--	--				B		

APPENDIX D

Mitigation Analysis

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		
Project Description <i>2012 P.M. TT - Enter Way/KGB with Pork Chop - Alt 1</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	464	27	19	598	17
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	3	478	27	19	616	17
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LTR			LTR		
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	0	41	0	0	1
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	0	0	42	0	0	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			1			1
Lanes	0	0	1	0	0	1
Configuration			R			R

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LTR	LTR			R			R
v (vph)	3	19			42			1
C (m) (vph)	960	1070			581			489
v/c	0.00	0.02			0.07			0.00
95% queue length	0.01	0.05			0.23			0.01
Control Delay	8.8	8.4			11.7			12.4
LOS	A	A			B			B
Approach Delay	--	--	11.7			12.4		
Approach LOS	--	--	B			B		

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		
Project Description <i>2022 P.M. TT - Enter Way/KGB with Pork Chop - Alt 1</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	5	721	27	19	929	27
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	5	743	27	19	957	27
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	LT		TR	LT		TR
Upstream Signal		1			0	

Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	0	0	41	0	0	2
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	0	0	42	0	0	2
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			1			1
Lanes	0	0	1	0	0	1
Configuration			R			R

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT			R			R
v (vph)	5	19			42			2
C (m) (vph)	710	921			760			528
v/c	0.01	0.02			0.06			0.00
95% queue length	0.02	0.06			0.18			0.01
Control Delay	10.1	9.0			10.0+			11.8
LOS	B	A			B			B
Approach Delay	--	--	10.0+			11.8		
Approach LOS	--	--	B			B		

>

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 P.M. TT - Site Access/Palmer-Wasilla Hwy - Alt 1								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	564	0	0	1012	30		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	626	0	0	1124	33		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration		T			T	TR		
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	166		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	0	0	0	184		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								R
v (vph)								184
C (m) (vph)								464
v/c								0.40
95% queue length								1.87
Control Delay								17.8
LOS								C
Approach Delay	--	--				17.8		
Approach LOS	--	--				C		

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/20/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 P.M. TT - Site Access/Palmer-Wasilla Hwy - Alt 1								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	0	852	0	0	1572	30		
Peak-Hour Factor, PHF	0.90	0.90			0.90	0.90		
Hourly Flow Rate, HFR	0	946	0	0	1746	33		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration		T			T	TR		
Upstream Signal		1			1			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	166		
Peak-Hour Factor, PHF						0.90		
Hourly Flow Rate, HFR	0	0	0	0	0	184		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								R
v (vph)								184
C (m) (vph)								638
v/c								0.29
95% queue length								1.19
Control Delay								12.9
LOS								B
Approach Delay	--	--				12.9		
Approach LOS	--	--				B		

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		
Project Description <i>2012 P.M. TT - Enter Way/Knik-Goose Bay Road TWLTL</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	464	27	19	598	17
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	3	478	27	19	616	17
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Two Way Left Turn Lane</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LTR</i>			<i>LTR</i>		
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	45	2	41	15	1	1
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	46	2	42	15	1	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LTR</i>	<i>LTR</i>		<i>LTR</i>			<i>LTR</i>	
v (vph)	3	19		90			17	
C (m) (vph)	960	1070		386			288	
v/c	0.00	0.02		0.23			0.06	
95% queue length	0.01	0.05		0.89			0.19	
Control Delay	8.8	8.4		17.1			18.3	
LOS	A	A		C			C	
Approach Delay	--	--	17.1			18.3		
Approach LOS	--	--	C			C		

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		
Project Description <i>2022 P.M. TT - Enter Way/Knik-Goose Bay Road TWLTL</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	5	721	27	19	929	27
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	5	743	27	19	957	27
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Two Way Left Turn Lane</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>TR</i>
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	45	2	41	24	1	2
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	46	2	42	24	1	2
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>		<i>LTR</i>			<i>LTR</i>	
v (vph)	5	19		90			27	
C (m) (vph)	710	938		397			219	
v/c	0.01	0.02		0.23			0.12	
95% queue length	0.02	0.06		0.86			0.41	
Control Delay	10.1	8.9		16.7			23.7	
LOS	<i>B</i>	<i>A</i>		<i>C</i>			<i>C</i>	
Approach Delay	--	--		16.7			23.7	
Approach LOS	--	--		<i>C</i>			<i>C</i>	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		
Project Description <i>2012 P.M. TT - Enter Way/Knik-Goose Bay Road With Turn Lanes</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	479	28	19	598	17
Peak-Hour Factor, PHF	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>
Hourly Flow Rate, HFR	3	493	28	19	616	17
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	1	1	1	0
Configuration	<i>LT</i>		<i>R</i>	<i>L</i>		<i>TR</i>
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	45	2	41	0	0	1
Peak-Hour Factor, PHF	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>
Hourly Flow Rate, HFR	46	2	42	0	0	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>L</i>		<i>LTR</i>			<i>LTR</i>	
v (vph)	3	19		90			1	
C (m) (vph)	960	1056		256			489	
v/c	0.00	0.02		0.35			0.00	
95% queue length	0.01	0.05		1.52			0.01	
Control Delay	8.8	8.5		26.5			12.4	
LOS	A	A		D			B	
Approach Delay	--	--	26.5			12.4		
Approach LOS	--	--	D			B		

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		
Project Description <i>2022 P.M. TT - Enter Way/Knik-Goose Bay Road with Turn Lanes</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	5	745	28	19	929	27
Peak-Hour Factor, PHF	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>
Hourly Flow Rate, HFR	5	768	28	19	957	27
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	1	1	2	0
Configuration	<i>LT</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>TR</i>
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	45	2	41	0	0	2
Peak-Hour Factor, PHF	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>	<i>0.97</i>
Hourly Flow Rate, HFR	46	2	42	0	0	2
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>L</i>		<i>LTR</i>			<i>LTR</i>	
v (vph)	5	19		90			2	
C (m) (vph)	710	888		214			528	
v/c	0.01	0.02		0.42			0.00	
95% queue length	0.02	0.07		1.94			0.01	
Control Delay	10.1	9.1		33.5			11.8	
LOS	<i>B</i>	<i>A</i>		<i>D</i>			<i>B</i>	
Approach Delay	--	--	33.5			11.8		
Approach LOS	--	--	<i>D</i>			<i>B</i>		

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		
Project Description <i>2012 A.M. Total Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	0	497	58	29	230	12
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	0	584	68	34	270	14
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LTR</i>			<i>LTR</i>		
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	7	0	16	4	0	1
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	8	0	18	4	0	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LTR</i>	<i>LTR</i>		<i>LTR</i>			<i>LTR</i>	
v (vph)	0	34		26			5	
C (m) (vph)	1290	940		392			266	
v/c	0.00	0.04		0.07			0.02	
95% queue length	0.00	0.11		0.21			0.06	
Control Delay	7.8	9.0		14.8			18.8	
LOS	<i>A</i>	<i>A</i>		<i>B</i>			<i>C</i>	
Approach Delay	--	--	14.8			18.8		
Approach LOS	--	--	<i>B</i>			<i>C</i>		

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2012</i>
Analysis Time Period	<i>2012</i>		
Project Description <i>2012 P.M. Total Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	3	464	12	19	598	17
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	3	478	12	19	616	17
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LTR			LTR		
Upstream Signal		1			0	

Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	45	2	41	15	1	1
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	46	2	42	15	1	1
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		LTR			LTR	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LTR	LTR		LTR			LTR	
v (vph)	3	19		90			17	
C (m) (vph)	960	1085		256			161	
v/c	0.00	0.02		0.35			0.11	
95% queue length	0.01	0.05		1.52			0.35	
Control Delay	8.8	8.4		26.5			30.0	
LOS	A	A		D			D	
Approach Delay	--	--		26.5			30.0	
Approach LOS	--	--		D			D	

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TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		
Project Description <i>2022 A.M. Total Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	0	772	58	29	358	19
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	0	908	68	34	421	22
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	LT		TR	LT		TR
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	7	0	16	7	0	2
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.85	0.85	0.85
Hourly Flow Rate, HFR	8	0	18	8	0	2
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			2		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		LTR			LTR	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT		LTR			LTR	
v (vph)	0	34		26			10	
C (m) (vph)	1128	790		359			310	
v/c	0.00	0.04		0.07			0.03	
95% queue length	0.00	0.13		0.23			0.10	
Control Delay	8.2	9.8		15.8			17.0	
LOS	A	A		C			C	
Approach Delay	--	--		15.8			17.0	
Approach LOS	--	--		C			C	

>

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	<i>Adam McGill</i>	Intersection	
Agency/Co.	<i>DOWL HKM</i>	Jurisdiction	<i>DOT&PF</i>
Date Performed	<i>1/12/2011</i>	Analysis Year	<i>2022</i>
Analysis Time Period	<i>2022</i>		
Project Description <i>2022 P.M. Total Traffic - Enter Way/Knik-Goose Bay Road</i>			
East/West Street: <i>Enter Way</i>		North/South Street: <i>Knik-Goose Bay Road</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume	5	721	12	19	929	27
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	5	743	12	19	957	27
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	LT		TR	LT		TR
Upstream Signal		1			0	
Minor Street	Westbound			Eastbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	45	2	41	24	1	2
Peak-Hour Factor, PHF	0.97	0.97	0.97	0.97	0.97	0.97
Hourly Flow Rate, HFR	46	2	42	24	1	2
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		LTR			LTR	

Delay, Queue Length, and Level of Service								
Approach	NB	SB	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT		LTR			LTR	
v (vph)	5	19		90			27	
C (m) (vph)	710	939		227			116	
v/c	0.01	0.02		0.40			0.23	
95% queue length	0.02	0.06		1.78			0.85	
Control Delay	10.1	8.9		30.9			45.2	
LOS	B	A		D			E	
Approach Delay	--	--		30.9			45.2	
Approach LOS	--	--		D			E	

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Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011



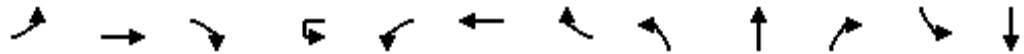
Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	13	26	173	37	5	561	661	35	204	21
v/c Ratio	0.10	0.13	0.44	0.13	0.01	0.50	0.55	0.07	0.18	0.02
Control Delay	31.7	26.6	30.7	13.1	6.0	12.4	3.2	6.0	7.7	4.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	31.7	26.6	30.7	13.1	6.0	12.4	3.2	6.0	7.7	4.7
Queue Length 50th (ft)	4	6	23	2	1	62	0	3	18	0
Queue Length 95th (ft)	22	31	67	27	5	294	55	17	95	11
Internal Link Dist (ft)		387		251		808			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	132	502	398	537	774	1111	1211	478	1161	994
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.05	0.43	0.07	0.01	0.50	0.55	0.07	0.18	0.02

Intersection Summary

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗			↖	↗		↖	↗	↖	↗	↗
Volume (vph)	12	21	3	21	138	7	27	5	516	608	32	188
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00			0.97	1.00		1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.98			1.00	0.88		1.00	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00			0.95	1.00		0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1831			3433	1644		1770	1863	1583	1770	1863
Flt Permitted	0.95	1.00			0.95	1.00		0.63	1.00	1.00	0.33	1.00
Satd. Flow (perm)	1770	1831			3433	1644		1173	1863	1583	617	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	13	23	3	23	150	8	29	5	561	661	35	204
RTOR Reduction (vph)	0	3	0	0	0	25	0	0	0	304	0	0
Lane Group Flow (vph)	13	23	0	0	173	12	0	5	561	357	35	204
Turn Type	Prot			Prot	Prot			pm+pt		Perm	pm+pt	
Protected Phases	7	4		3	3	8		5	2			1
Permitted Phases								2		2		6
Actuated Green, G (s)	0.8	3.5			7.1	9.8		38.5	37.7	37.7	40.1	38.5
Effective Green, g (s)	0.8	3.5			7.1	9.8		38.5	37.7	37.7	40.1	38.5
Actuated g/C Ratio	0.01	0.05			0.10	0.14		0.55	0.54	0.54	0.57	0.55
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	20	92			349	230		653	1005	854	380	1026
v/s Ratio Prot	0.01	c0.01			c0.05	0.01		0.00	c0.30		c0.00	0.11
v/s Ratio Perm								0.00		0.23	0.05	
v/c Ratio	0.65	0.25			0.50	0.05		0.01	0.56	0.42	0.09	0.20
Uniform Delay, d1	34.4	31.9			29.7	26.0		7.1	10.6	9.6	7.3	7.9
Progression Factor	1.00	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	56.6	1.4			1.1	0.1		0.0	2.2	1.5	0.1	0.4
Delay (s)	91.0	33.4			30.8	26.1		7.1	12.8	11.1	7.4	8.4
Level of Service	F	C			C	C		A	B	B	A	A
Approach Delay (s)		52.6				30.0			11.9			8.1
Approach LOS		D				C			B			A

Intersection Summary

HCM Average Control Delay	14.4	HCM Level of Service	B
HCM Volume to Capacity ratio	0.51		
Actuated Cycle Length (s)	69.9	Sum of lost time (s)	20.0
Intersection Capacity Utilization	64.7%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/19/2011

Movement	SBR
Lane Configurations	7
Volume (vph)	19
Ideal Flow (vphpl)	1900
Total Lost time (s)	5.0
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.92
Adj. Flow (vph)	21
RTOR Reduction (vph)	9
Lane Group Flow (vph)	12
Turn Type	Perm
Protected Phases	
Permitted Phases	6
Actuated Green, G (s)	38.5
Effective Green, g (s)	38.5
Actuated g/C Ratio	0.55
Clearance Time (s)	5.0
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	872
v/s Ratio Prot	
v/s Ratio Perm	0.01
v/c Ratio	0.01
Uniform Delay, d1	7.1
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	7.1
Level of Service	A
Approach Delay (s)	
Approach LOS	
Intersection Summary	

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/21/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	29	57	1071	158	11	374	450	86	596	20
v/c Ratio	0.21	0.34	0.83	0.21	0.07	0.61	0.55	0.29	0.83	0.03
Control Delay	49.7	41.5	33.0	7.1	23.8	36.9	6.0	26.1	41.4	16.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	49.7	41.5	33.0	7.1	23.8	36.9	6.0	26.1	41.4	16.0
Queue Length 50th (ft)	17	25	307	16	4	204	0	33	324	2
Queue Length 95th (ft)	51	71	407	56	19	#401	85	83	#772	23
Internal Link Dist (ft)		387		251		808			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	420	344	1941	925	164	611	822	293	720	621
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.17	0.55	0.17	0.07	0.61	0.55	0.29	0.83	0.03

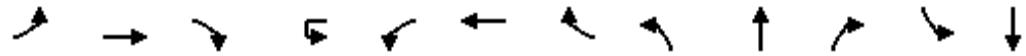
Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/21/2011



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗			↖	↗		↖	↗	↖	↗	↖
Volume (vph)	27	37	16	53	932	38	108	10	344	414	79	548
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00			0.97	1.00		1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.96			1.00	0.89		1.00	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00			0.95	1.00		0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1779			3433	1656		1770	1863	1583	1770	1863
Flt Permitted	0.95	1.00			0.95	1.00		0.13	1.00	1.00	0.32	1.00
Satd. Flow (perm)	1770	1779			3433	1656		239	1863	1583	591	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	29	40	17	58	1013	41	117	11	374	450	86	596
RTOR Reduction (vph)	0	14	0	0	0	72	0	0	0	298	0	0
Lane Group Flow (vph)	29	43	0	0	1071	86	0	11	374	152	86	596
Turn Type	Prot			Prot	Prot			pm+pt		Perm	pm+pt	
Protected Phases	7	4		3	3	8		5	2			6
Permitted Phases								2		2		6
Actuated Green, G (s)	4.4	7.3			35.1	38.0		34.1	33.4	33.4	39.3	36.0
Effective Green, g (s)	4.4	7.3			35.1	38.0		34.1	33.4	33.4	39.3	36.0
Actuated g/C Ratio	0.04	0.07			0.35	0.38		0.34	0.34	0.34	0.40	0.36
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	79	131			1216	635		93	628	534	274	677
v/s Ratio Prot	0.02	c0.02			c0.31	0.05		0.00	0.20		c0.01	c0.32
v/s Ratio Perm								0.04		0.10		0.11
v/c Ratio	0.37	0.33			0.88	0.14		0.12	0.60	0.28	0.31	0.88
Uniform Delay, d1	46.0	43.6			30.0	19.9		24.3	27.2	24.1	20.2	29.5
Progression Factor	1.00	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	2.9	1.5			7.7	0.1		0.6	4.1	1.3	0.7	15.2
Delay (s)	48.9	45.1			37.8	20.0		24.9	31.4	25.4	20.8	44.8
Level of Service	D	D			D	B		C	C	C	C	D
Approach Delay (s)		46.3				35.5			28.1			41.1
Approach LOS		D				D			C			D

Intersection Summary

HCM Average Control Delay	35.0	HCM Level of Service	D
HCM Volume to Capacity ratio	0.83		
Actuated Cycle Length (s)	99.1	Sum of lost time (s)	20.0
Intersection Capacity Utilization	79.4%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/21/2011

Movement	SBR
Lane Configurations	7
Volume (vph)	18
Ideal Flow (vphpl)	1900
Total Lost time (s)	5.0
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.92
Adj. Flow (vph)	20
RTOR Reduction (vph)	9
Lane Group Flow (vph)	11
Turn Type	Perm
Protected Phases	
Permitted Phases	6
Actuated Green, G (s)	36.0
Effective Green, g (s)	36.0
Actuated g/C Ratio	0.36
Clearance Time (s)	5.0
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	575
v/s Ratio Prot	
v/s Ratio Perm	0.01
v/c Ratio	0.02
Uniform Delay, d1	20.2
Progression Factor	1.00
Incremental Delay, d2	0.1
Delay (s)	20.3
Level of Service	C
Approach Delay (s)	
Approach LOS	
Intersection Summary	

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/21/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	18	39	254	57	9	837	983	53	314	32
v/c Ratio	0.16	0.24	0.63	0.19	0.01	0.39	0.76	0.13	0.14	0.03
Control Delay	44.6	38.4	44.5	15.5	6.5	11.0	7.6	6.8	7.3	3.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	44.6	38.4	44.5	15.5	6.5	11.0	7.6	6.8	7.3	3.6
Queue Length 50th (ft)	10	19	73	5	2	143	45	10	33	0
Queue Length 95th (ft)	32	50	#116	40	7	200	232	24	71	13
Internal Link Dist (ft)		387		251		589			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	113	368	419	458	710	2167	1287	418	2317	1047
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.11	0.61	0.12	0.01	0.39	0.76	0.13	0.14	0.03

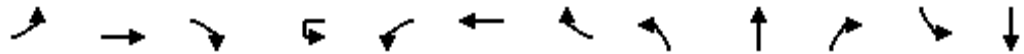
Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/21/2011



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗			↖	↗		↖	↗	↖	↗	↗
Volume (vph)	17	31	5	22	212	10	42	8	770	904	49	289
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00			0.97	1.00		1.00	0.95	1.00	1.00	0.95
Frt	1.00	0.98			1.00	0.88		1.00	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00			0.95	1.00		0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1827			3433	1637		1770	3539	1583	1770	3539
Flt Permitted	0.95	1.00			0.95	1.00		0.56	1.00	1.00	0.28	1.00
Satd. Flow (perm)	1770	1827			3433	1637		1045	3539	1583	525	3539
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	18	34	5	24	230	11	46	9	837	983	53	314
RTOR Reduction (vph)	0	5	0	0	0	39	0	0	0	340	0	0
Lane Group Flow (vph)	18	34	0	0	254	18	0	9	837	643	53	314
Turn Type	Prot			Prot	Prot			pm+pt		Perm	pm+pt	
Protected Phases	7	4		3	3	8		5	2			6
Permitted Phases								2		2		6
Actuated Green, G (s)	1.9	5.4			9.9	13.4		54.1	53.3	53.3	57.5	55.0
Effective Green, g (s)	1.9	5.4			9.9	13.4		54.1	53.3	53.3	57.5	55.0
Actuated g/C Ratio	0.02	0.06			0.11	0.15		0.59	0.59	0.59	0.63	0.60
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	37	108			373	241		627	2071	926	366	2137
v/s Ratio Prot	0.01	c0.02			c0.07	0.01		0.00	0.24		c0.00	0.09
v/s Ratio Perm								0.01		c0.41	0.09	
v/c Ratio	0.49	0.32			0.68	0.07		0.01	0.40	0.69	0.14	0.15
Uniform Delay, d1	44.1	41.1			39.1	33.5		7.6	10.3	13.2	6.9	7.8
Progression Factor	1.00	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	9.7	1.7			5.1	0.1		0.0	0.6	4.3	0.2	0.1
Delay (s)	53.8	42.8			44.1	33.6		7.6	10.9	17.5	7.1	8.0
Level of Service	D	D			D	C		A	B	B	A	A
Approach Delay (s)		46.3				42.2			14.4			7.8
Approach LOS		D				D			B			A

Intersection Summary

HCM Average Control Delay	17.4	HCM Level of Service	B
HCM Volume to Capacity ratio	0.64		
Actuated Cycle Length (s)	91.1	Sum of lost time (s)	20.0
Intersection Capacity Utilization	85.2%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/21/2011

Movement	SBR
Lane Configurations	
Volume (vph)	29
Ideal Flow (vphpl)	1900
Total Lost time (s)	5.0
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.92
Adj. Flow (vph)	32
RTOR Reduction (vph)	13
Lane Group Flow (vph)	19
Turn Type	Perm
Protected Phases	
Permitted Phases	6
Actuated Green, G (s)	55.0
Effective Green, g (s)	55.0
Actuated g/C Ratio	0.60
Clearance Time (s)	5.0
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	956
v/s Ratio Prot	
v/s Ratio Perm	0.01
v/c Ratio	0.02
Uniform Delay, d1	7.2
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	7.3
Level of Service	A
Approach Delay (s)	
Approach LOS	
Intersection Summary	

Queues

1: E Riley Avenue & Knik-Goose Bay Road

1/21/2011



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	45	88	1592	246	16	574	690	133	898	29
v/c Ratio	0.44	0.57	1.01	0.27	0.13	0.66	0.76	0.58	0.78	0.04
Control Delay	77.1	66.2	64.0	8.6	33.8	52.5	9.4	44.4	49.3	11.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	77.1	66.2	64.0	8.6	33.8	52.5	9.4	44.4	49.3	11.3
Queue Length 50th (ft)	40	67	~764	47	9	255	0	84	371	2
Queue Length 95th (ft)	83	121	#923	96	28	323	129	#157	#581	25
Internal Link Dist (ft)		387		563		589			818	
Turn Bay Length (ft)	140		335		250		250	240		240
Base Capacity (vph)	115	215	1569	922	127	869	909	231	1149	689
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.39	0.41	1.01	0.27	0.13	0.66	0.76	0.58	0.78	0.04

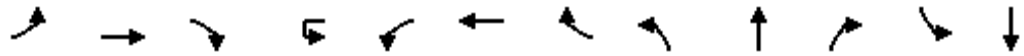
Intersection Summary

- ~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/21/2011



Movement	EBL	EBT	EBR	WBU	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT
Lane Configurations	↖	↗			↖	↗		↖	↑↑	↗	↖	↑↑
Volume (vph)	41	56	25	58	1407	58	168	15	528	635	122	826
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Lane Util. Factor	1.00	1.00			0.97	1.00		1.00	0.95	1.00	1.00	0.95
Frt	1.00	0.95			1.00	0.89		1.00	1.00	0.85	1.00	1.00
Flt Protected	0.95	1.00			0.95	1.00		0.95	1.00	1.00	0.95	1.00
Satd. Flow (prot)	1770	1777			3433	1655		1770	3539	1583	1770	3539
Flt Permitted	0.95	1.00			0.95	1.00		0.12	1.00	1.00	0.19	1.00
Satd. Flow (perm)	1770	1777			3433	1655		222	3539	1583	359	3539
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	45	61	27	63	1529	63	183	16	574	690	133	898
RTOR Reduction (vph)	0	12	0	0	0	76	0	0	0	525	0	0
Lane Group Flow (vph)	45	76	0	0	1592	170	0	16	574	165	133	898
Turn Type	Prot			Prot	Prot			pm+pt		Prot	pm+pt	
Protected Phases	7	4		3	3	8		5	2	2		6
Permitted Phases								2				6
Actuated Green, G (s)	6.9	12.2			64.0	69.3		35.8	33.5	33.5	48.8	41.5
Effective Green, g (s)	6.9	12.2			64.0	69.3		35.8	33.5	33.5	48.8	41.5
Actuated g/C Ratio	0.05	0.09			0.46	0.49		0.26	0.24	0.24	0.35	0.30
Clearance Time (s)	5.0	5.0			5.0	5.0		5.0	5.0	5.0	5.0	5.0
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	87	155			1569	819		82	847	379	229	1049
v/s Ratio Prot	0.03	c0.04			c0.46	0.10		0.00	0.16	0.10	c0.04	c0.25
v/s Ratio Perm								0.05			0.16	
v/c Ratio	0.52	0.49			1.01	0.21		0.20	0.68	0.44	0.58	0.86
Uniform Delay, d1	64.9	60.9			38.0	19.9		40.9	48.3	45.2	34.1	46.4
Progression Factor	1.00	1.00			1.00	1.00		1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	5.1	2.4			26.4	0.1		1.2	4.3	3.6	3.7	9.0
Delay (s)	70.0	63.4			64.4	20.0		42.1	52.7	48.8	37.8	55.4
Level of Service	E	E			E	C		D	D	D	D	E
Approach Delay (s)		65.6				58.5			50.5			52.5
Approach LOS		E				E			D			D

Intersection Summary

HCM Average Control Delay	54.9	HCM Level of Service	D
HCM Volume to Capacity ratio	0.91		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	20.0
Intersection Capacity Utilization	107.0%	ICU Level of Service	G
Analysis Period (min)	15		
c Critical Lane Group			

HCM Signalized Intersection Capacity Analysis

1: E Riley Avenue & Knik-Goose Bay Road

1/21/2011



Movement	SBR
Lane Configurations	7
Volume (vph)	27
Ideal Flow (vphpl)	1900
Total Lost time (s)	5.0
Lane Util. Factor	1.00
Frt	0.85
Flt Protected	1.00
Satd. Flow (prot)	1583
Flt Permitted	1.00
Satd. Flow (perm)	1583
Peak-hour factor, PHF	0.92
Adj. Flow (vph)	29
RTOR Reduction (vph)	17
Lane Group Flow (vph)	12
Turn Type	pm+ov
Protected Phases	7
Permitted Phases	6
Actuated Green, G (s)	48.4
Effective Green, g (s)	48.4
Actuated g/C Ratio	0.35
Clearance Time (s)	5.0
Vehicle Extension (s)	3.0
Lane Grp Cap (vph)	604
v/s Ratio Prot	0.00
v/s Ratio Perm	0.01
v/c Ratio	0.02
Uniform Delay, d1	30.2
Progression Factor	1.00
Incremental Delay, d2	0.0
Delay (s)	30.2
Level of Service	C
Approach Delay (s)	
Approach LOS	
Intersection Summary	

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 A.M. Total Traffic - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	75	613	0	0	165	20		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	81	666	0	0	179	21		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration	LT				T	TR		
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	24		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	0	0	0	26		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT							R
v (vph)	81							26
C (m) (vph)	1384							943
v/c	0.06							0.03
95% queue length	0.19							0.08
Control Delay	7.8							8.9
LOS	A							A
Approach Delay	--	--				8.9		
Approach LOS	--	--				A		

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2012					
Analysis Time Period	2012							
Project Description 2012 P.M. Total Traffic - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	14	564	0	0	1012	30		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	15	626	0	0	1124	33		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration	LT				T	TR		
Upstream Signal		1			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	119		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90		
Hourly Flow Rate, HFR	0	0	0	0	0	132		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT							R
v (vph)	15							132
C (m) (vph)	611							464
v/c	0.02							0.28
95% queue length	0.08							1.16
Control Delay	11.0							15.8
LOS	B							C
Approach Delay	--	--				15.8		
Approach LOS	--	--				C		

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 A.M. Total Traffic - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	75	944	0	0	263	20		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	81	1026	0	0	285	21		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration	LT				T	TR		
Upstream Signal		1			1			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	24		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly Flow Rate, HFR	0	0	0	0	0	26		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT							R
v (vph)	81							26
C (m) (vph)	1266							872
v/c	0.06							0.03
95% queue length	0.20							0.09
Control Delay	8.0							9.3
LOS	A							A
Approach Delay	--	--				9.3		
Approach LOS	--	--				A		

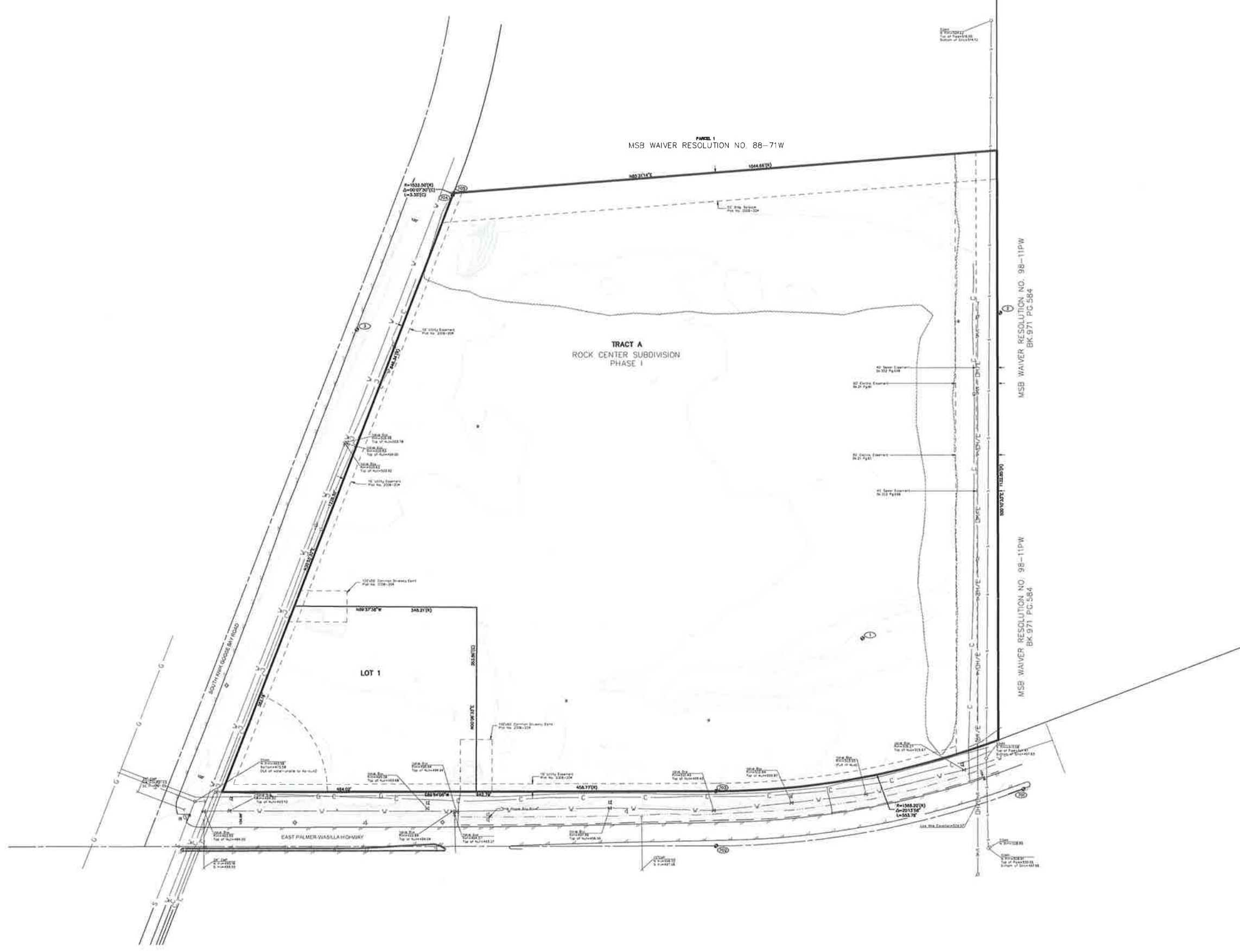
TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	Adam McGill	Intersection						
Agency/Co.	DOWL HKM	Jurisdiction	DOT&PF					
Date Performed	1/12/2011	Analysis Year	2022					
Analysis Time Period	2022							
Project Description 2022 P.M. Total Traffic - Site Access/Palmer-Wasilla Hwy								
East/West Street: Palmer-Wasilla Highway			North/South Street: Site Access					
Intersection Orientation: East-West			Study Period (hrs): 0.25					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume	14	852	0	0	1572	30		
Peak-Hour Factor, PHF	0.90	0.90			0.90	0.90		
Hourly Flow Rate, HFR	15	946	0	0	1746	33		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	2	0		
Configuration	LT				T	TR		
Upstream Signal		1			1			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume	0	0	0	0	0	119		
Peak-Hour Factor, PHF						0.90		
Hourly Flow Rate, HFR	0	0	0	0	0	132		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	2			-2				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	1		
Configuration						R		
Delay, Queue Length, and Level of Service								
Approach	EB	WB	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT							R
v (vph)	15							132
C (m) (vph)	197							638
v/c	0.08							0.21
95% queue length	0.24							0.77
Control Delay	24.8							12.1
LOS	C							B
Approach Delay	--	--				12.1		
Approach LOS	--	--				B		



APPENDIX C

Civil Site Plan



kpb architects

nbbj

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DOWL-HKM

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Wasilla, Alaska 99684
Phone: (907) 276-1058
Fax: (907) 276-1058

**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

#	REVISION	DATE

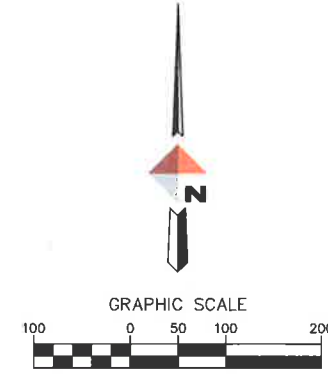
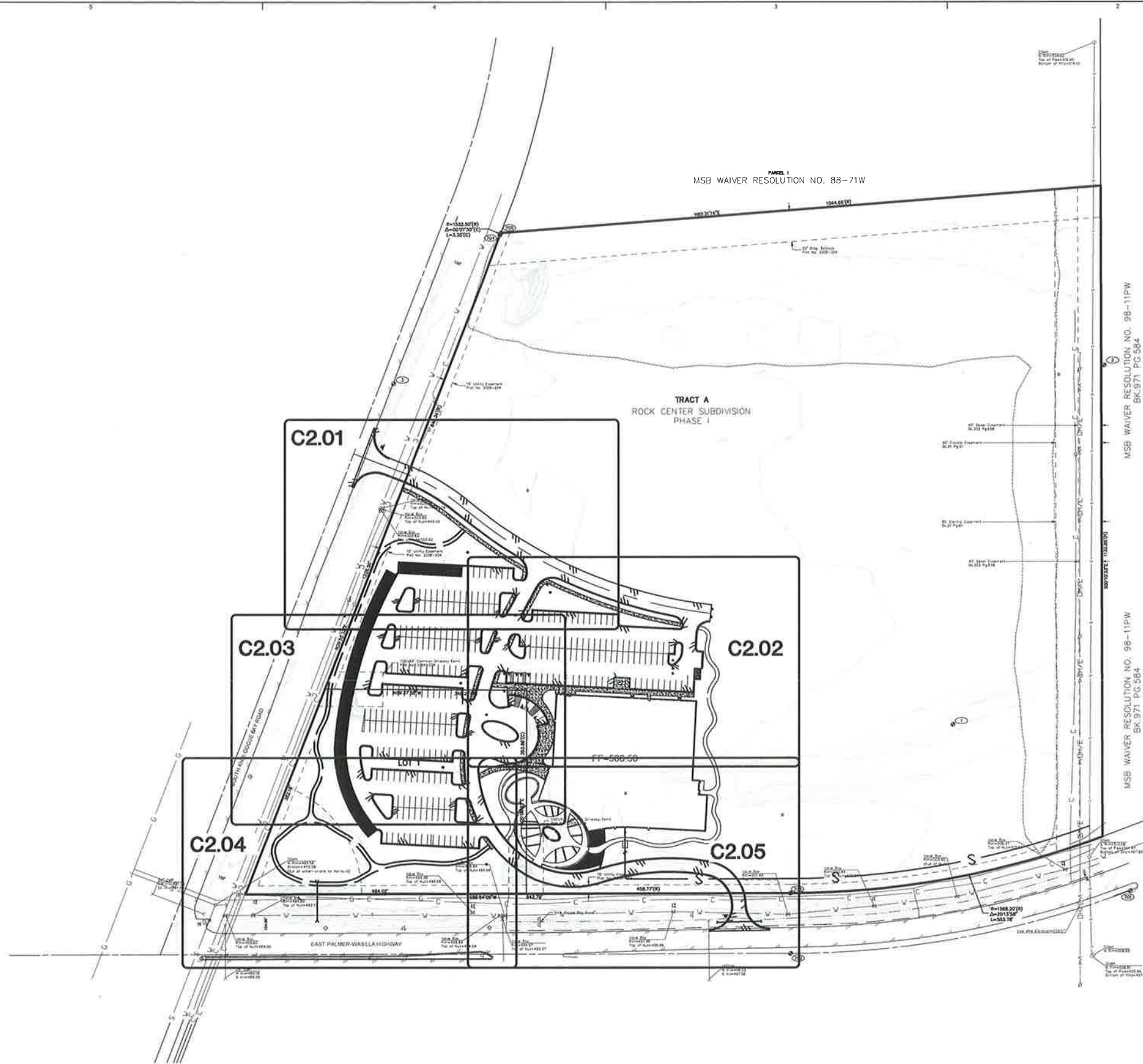
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JOB NO. - kpb	A0061.01
JOB NO. - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
OVERALL EXISTING
CONDITIONS PLAN

SHEET NO
C1.00

CONDITION USE PERMIT - DRAFT

PAGE SIZE: 22" x 34" - 1/4" x 1/4" BEE 11" x 17"



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

#	REVISION	SCHEDULE	DESCRIPTION	DATE

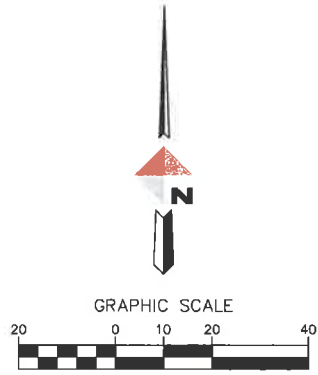
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JOB NO. - kpb	A9061.01
JOB NO. - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME	OVERALL SITE PLAN
SHEET NO.	C2.00

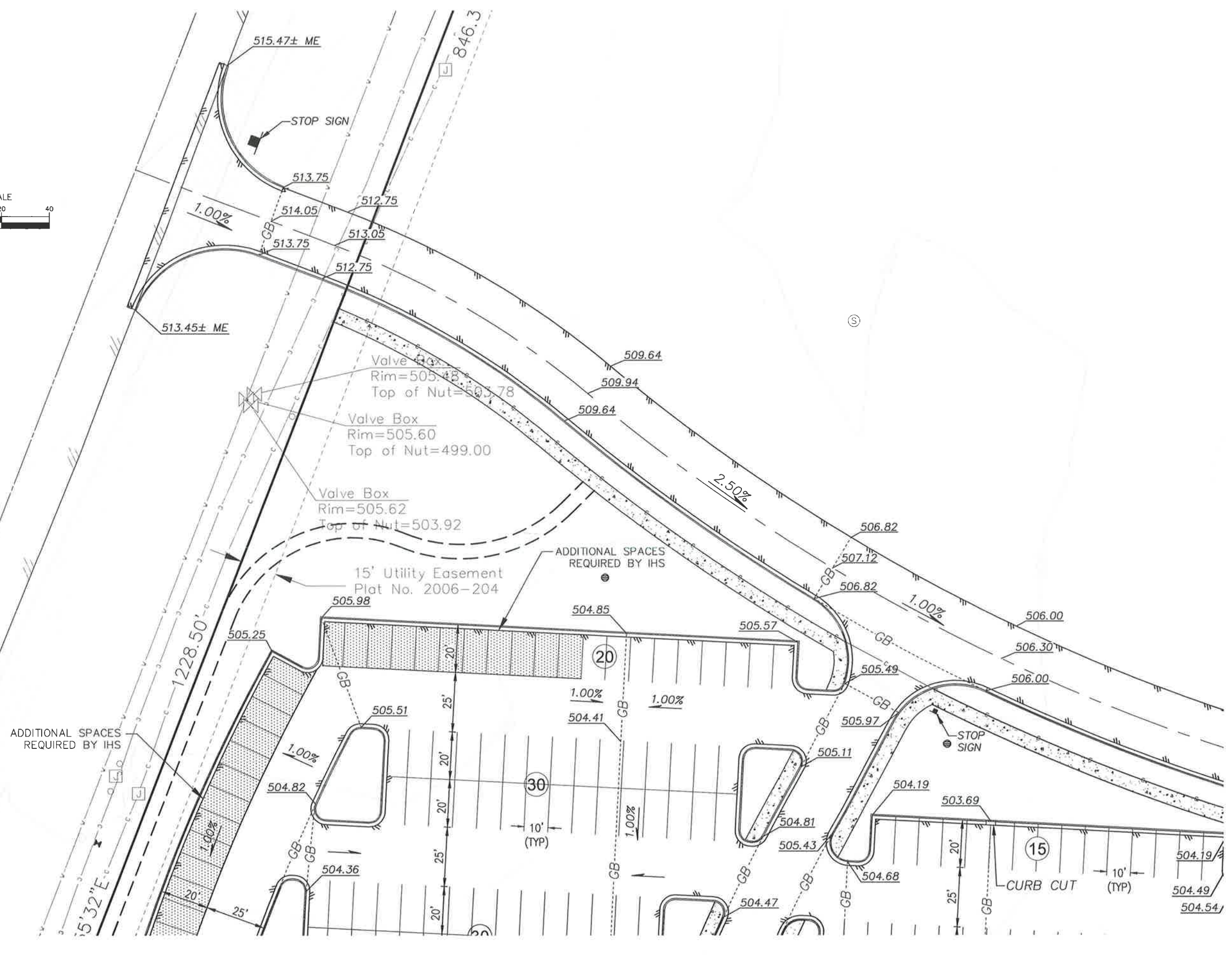
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SOUTH CENTRAL FOUNDATION NATIVE PRIMARY CARE CLINIC WASILLA, AK

REVISION	SCHEDULE	DESCRIPTION	DATE

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JOB NO. - kpb	A0001.01
JOB NO. - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

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SHEET NO.	C2.01

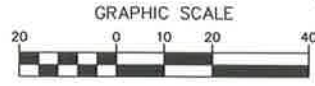
CONDITION USE PERMIT - DRAFT

FULL SIZE 22"X34" - HALF SIZE 11"X17"

DRAWN FILE NO.

SCRIPT FILE

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KNIK GOOSE BAY ROAD

1
C2.06

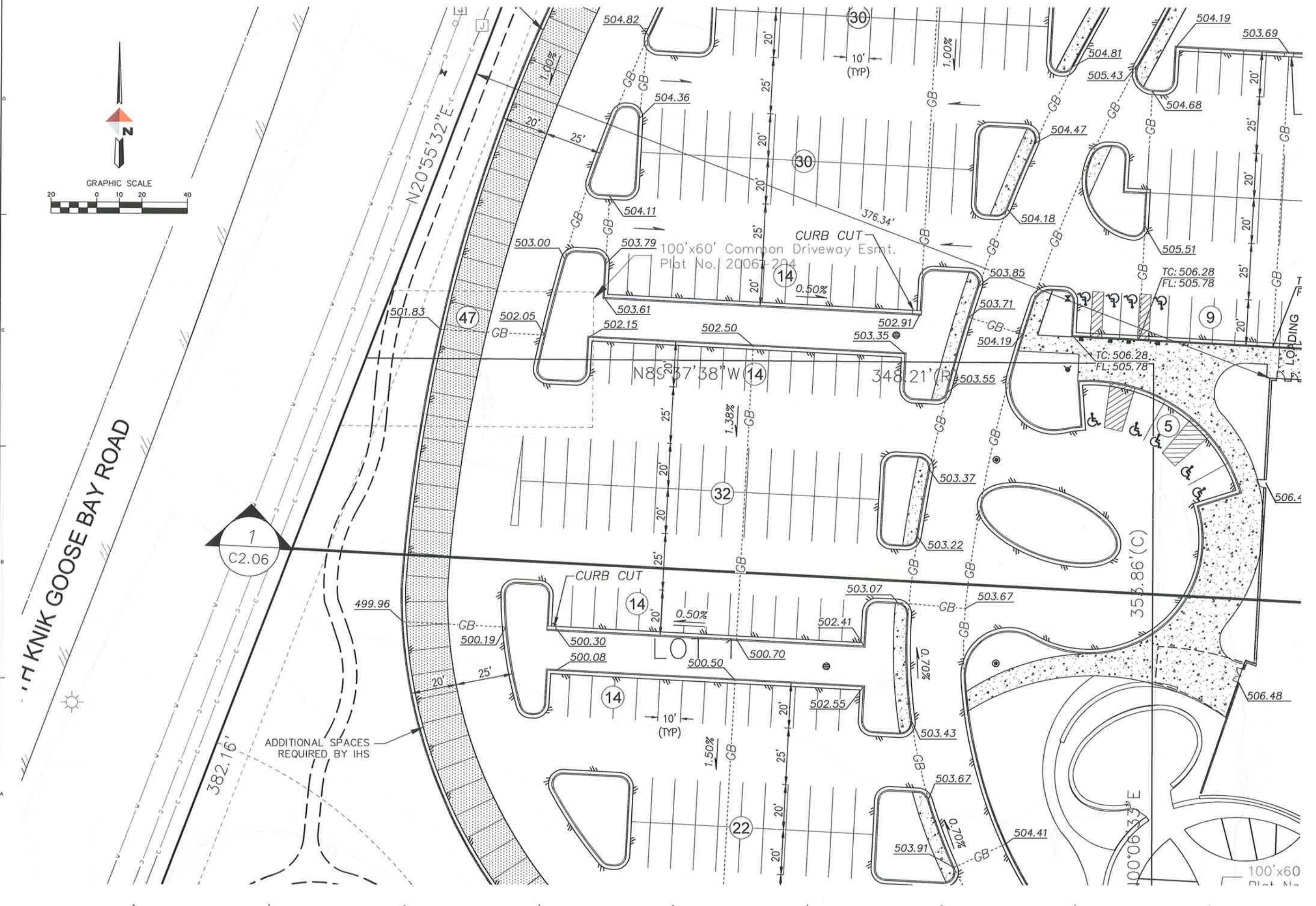
ADDITIONAL SPACES
REQUIRED BY IHS

382.16'

N20°55'32"E

N85°37'38"W

100'x60'
Plot No.



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

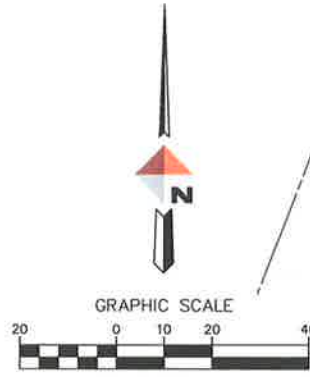
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JOB NO. - kpb	A9061 01
JOB NO. - nbbj	100748 00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH
SHEET NAME	SITE GRADING PLAN
SHEET NO.	C2.03

CONDITION: USE PERMIT - DRAFT

FULL SIZE 32x44" - HALF SIZE 11x17"

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SOUTH KN

ADDITIONAL SPACES
REQUIRED BY IHS

SSMH
N Rim=493.58
Bottom=475.58
(full of water-unable to As-built)

Valve Box
Rim=494.80
Top of Nut=493.10

Valve Box
Rim=493.60
Top of Nut=484.00

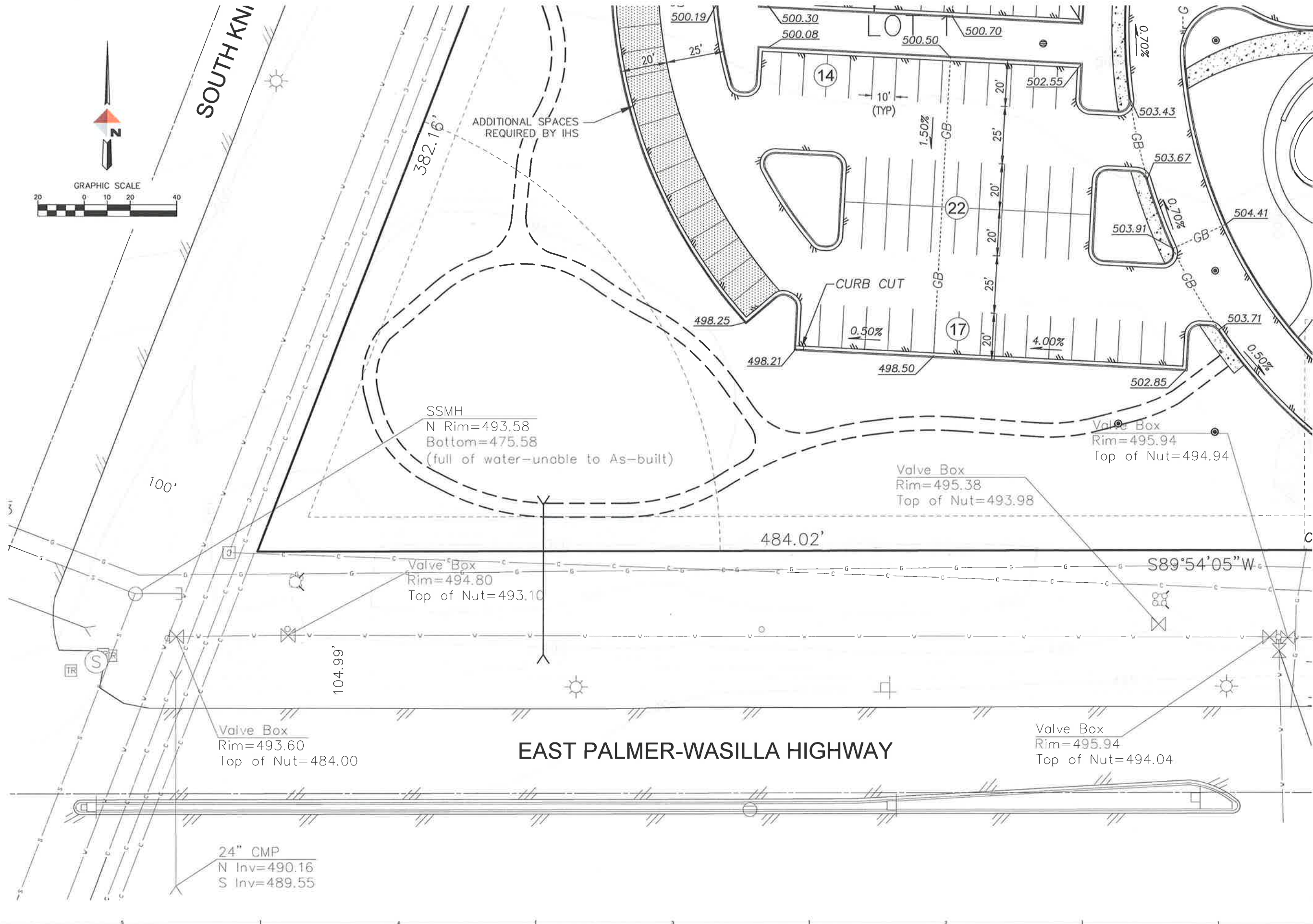
24" CMP
N Inv=490.16
S Inv=489.55

EAST PALMER-WASILLA HIGHWAY

Valve Box
Rim=495.38
Top of Nut=493.98

Valve Box
Rim=495.94
Top of Nut=494.94

Valve Box
Rim=495.94
Top of Nut=494.04



kpb architects

nbbj

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NEESER CONSTRUCTION, INC.

2525 Bankers Road
 Corra, AK 99574
 (907) 274-1058

Anchorage, Alaska 99505
 Fax (907) 274-8823



**SOUTH CENTRAL FOUNDATION
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#	REVISIONS	DATE

JOB NO - NCI	1101
JOB NO - kpb	A9061.01
JOB NO - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

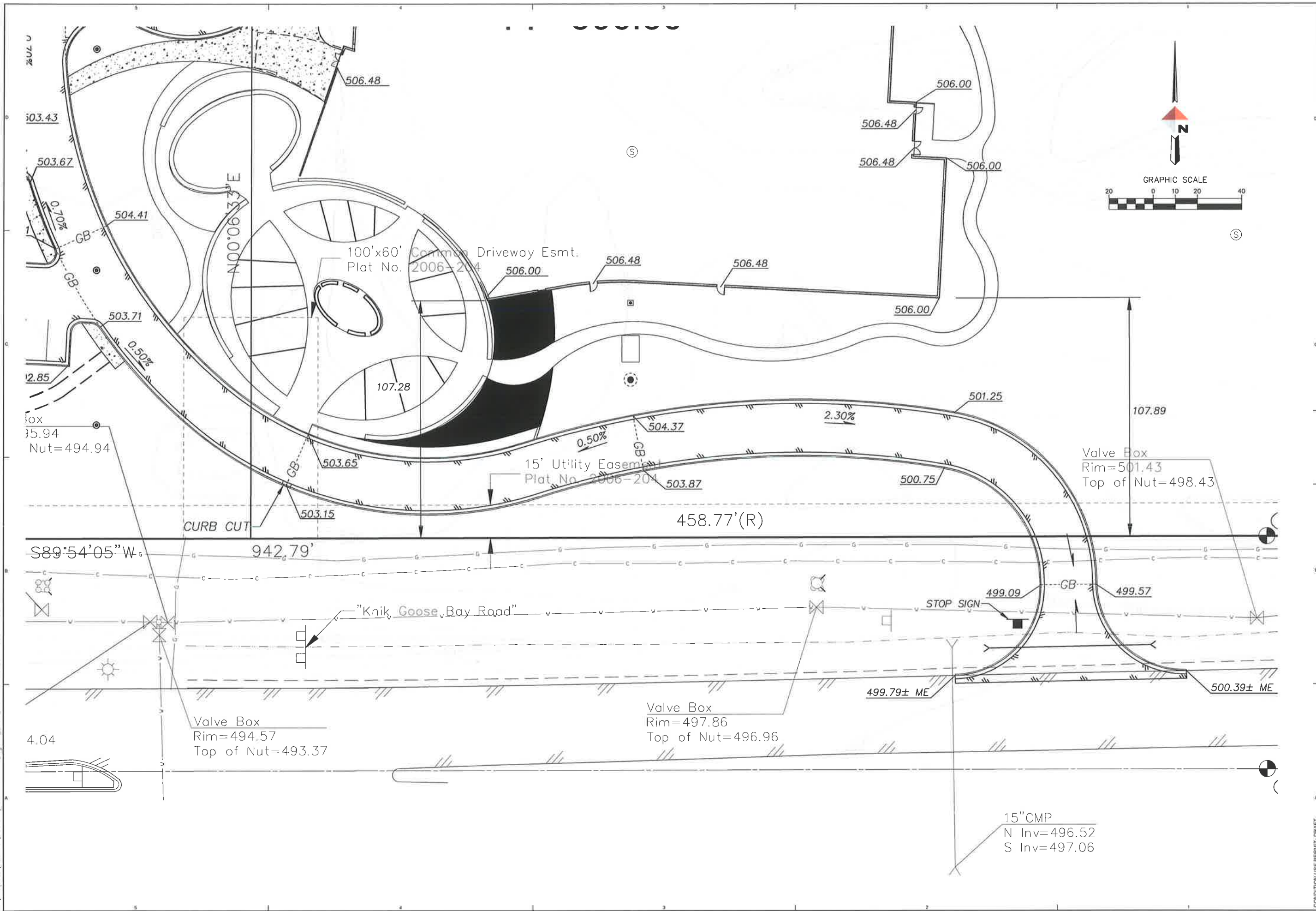
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SITE GRADING PLAN

SHEET NO
C2.04

CONDITION USE PERMIT - DRAFT

PULL SEE 22'x34' - HALF SEE 11'x17'

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**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

NO.	REVISION	DATE

JOB NO - HCI	11101
JOB NO - kpb	A9061 01
JOB NO - nbbj	100748 00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
SITE GRADING PLAN

SHEET NO.
C2.05

FULL SIZE 22"X34" - HALF SIZE 11"X17"

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PROPERTY LINE

EASEMENT

SEE LANDSCAPING FOR SURFACE MATERIAL

ARFS

3

1

1

MINIMUM 3.3' CLASSIFIED MATERIAL @ 95% COMPACTION

2" AC PAVEMENT

2" LC

PROPOSED GRADE

EXISTING GROUND

2" AC PAVEMENT

2" LC

PROPOSED GRADE

EXISTING GROUND

VARIES CONCRETE SIDEWALK

2% MAX

FF=506.5

LIMIT OF EXCAVATION TO REMOVE ORGANICS AND UNSUITABLE SOILS

EXISTING GROUND

LIMIT OF EXCAVATION TO REMOVE ORGANICS AND UNSUITABLE SOILS

FF=506.5

3

1

SEE LANDSCAPING FOR SURFACE MATERIAL

ARFS

1
C2.07

WEST TO EAST SITE SECTION PHASE 1

NTS



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

REVISION	DESCRIPTION	DATE

JOB NO - NCI	11101
JOB NO - kpb	A9061.01
JOB NO - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
SITE SECTIONS

SHEET NO.
C2.06

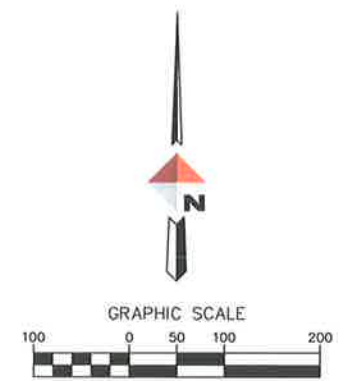
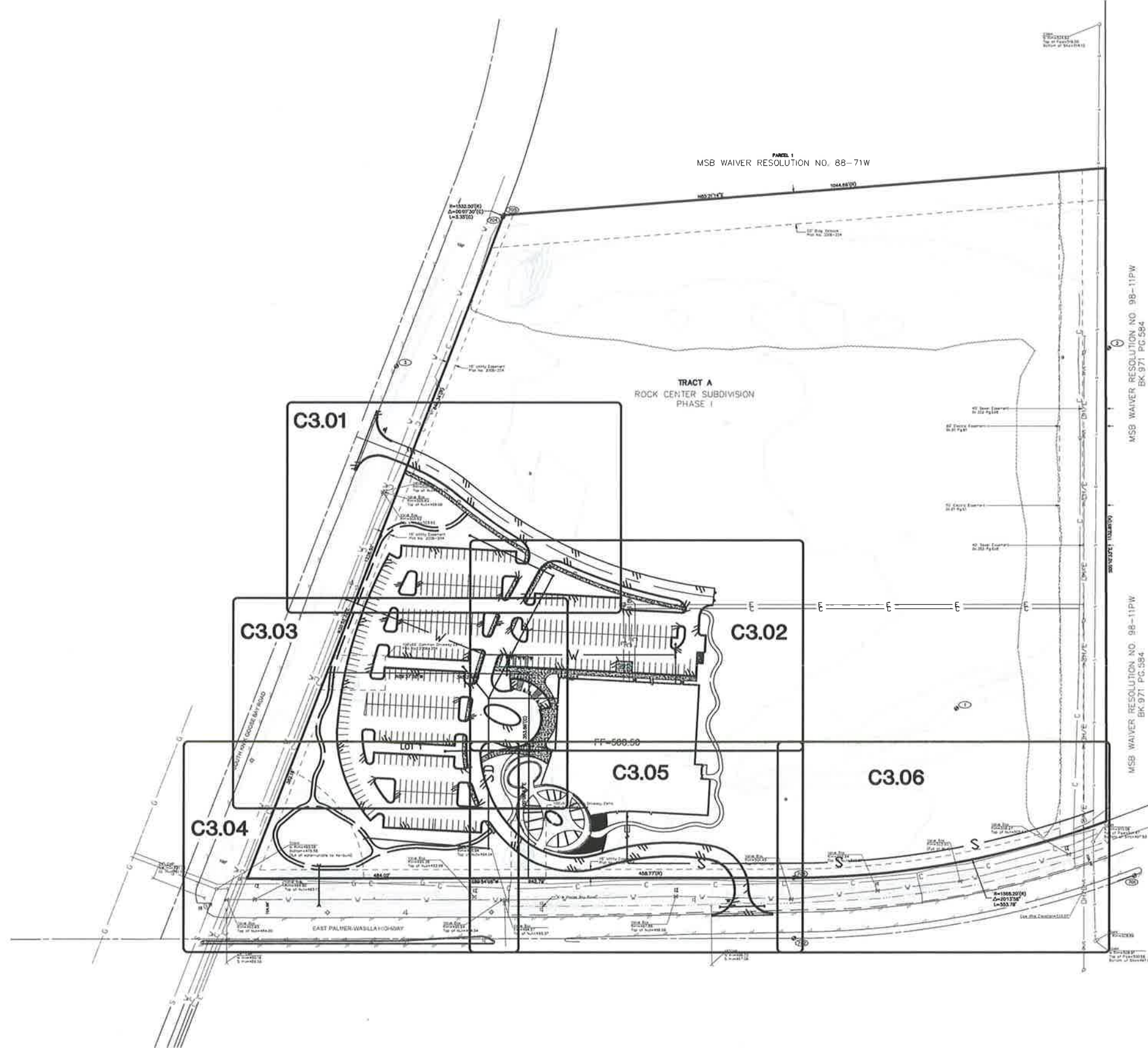
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PULL SEE 22' OF - HALF SHEET 11X17'

DOWL FILE NO:

SCRIPT FILE:

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kpb architects
nbbj
 233 13th Avenue North, Suite 100, Wasilla, AK 99587
 (907) 274-8333

NEESER CONSTRUCTION, INC.
 Anchorage, Alaska 99503
 5155 Brainerd Road
 Circle (907) 374-0298
 Fax (907) 374-8333

DOWL HKM
 2011-2-28 14:27:57

**SOUTH CENTRAL FOUNDATION
 NATIVE PRIMARY CARE CLINIC
 WASILLA, AK**

REVISIONS SCHEDULE	
#	DESCRIPTION

JOB NO - NCI	11101
JOB NO - kpb	A0061.01
JOB NO - nbbj	100749.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH
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OVERALL SITE UTILITY PLAN	
SHEET NO	
C3.00	

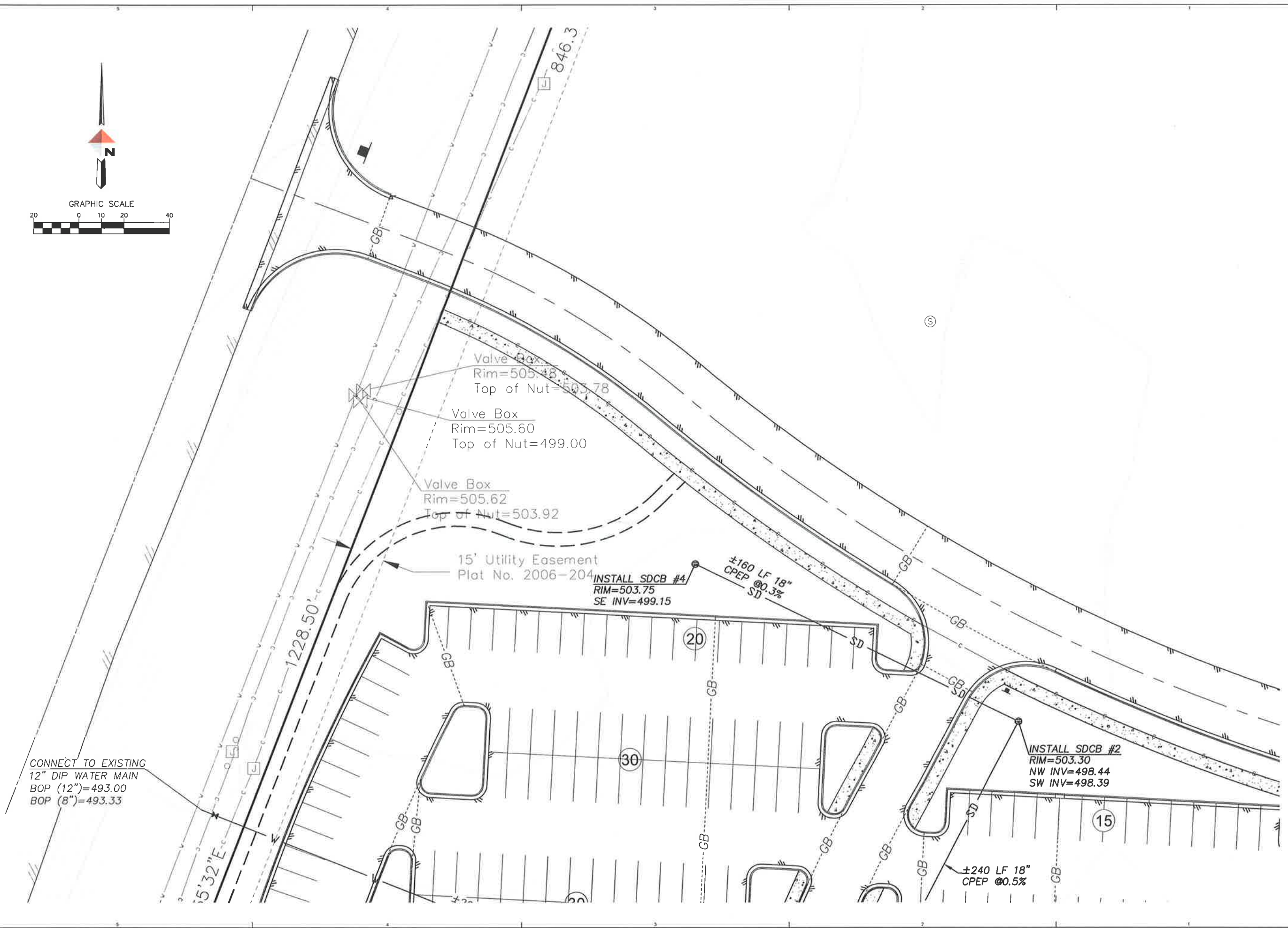
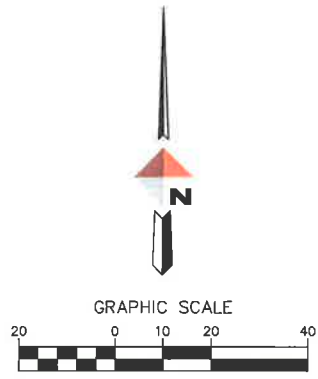
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FULL SIZE 22"x34" HALF SIZE 11"x17"

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SCRIPT FILE

DOWL FILE No:



CONNECT TO EXISTING
12" DIP WATER MAIN
BOP (12")=493.00
BOP (8")=493.33

Valve Box
Rim=505.48
Top of Nut=505.78

Valve Box
Rim=505.60
Top of Nut=499.00

Valve Box
Rim=505.62
Top of Nut=503.92

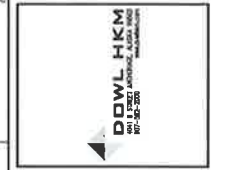
15' Utility Easement
Plat No. 2006-204

INSTALL SDCB #4
RIM=503.75
SE INV=499.15

±160 LF 18"
CPEP @0.3%
SD

INSTALL SDCB #2
RIM=503.30
NW INV=498.44
SW INV=498.39

±240 LF 18"
CPEP @0.5%
SD



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

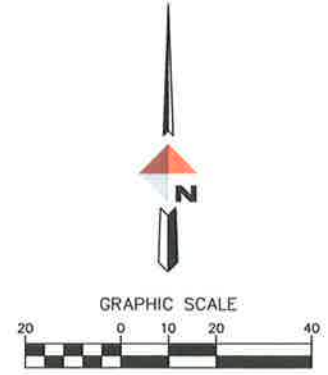
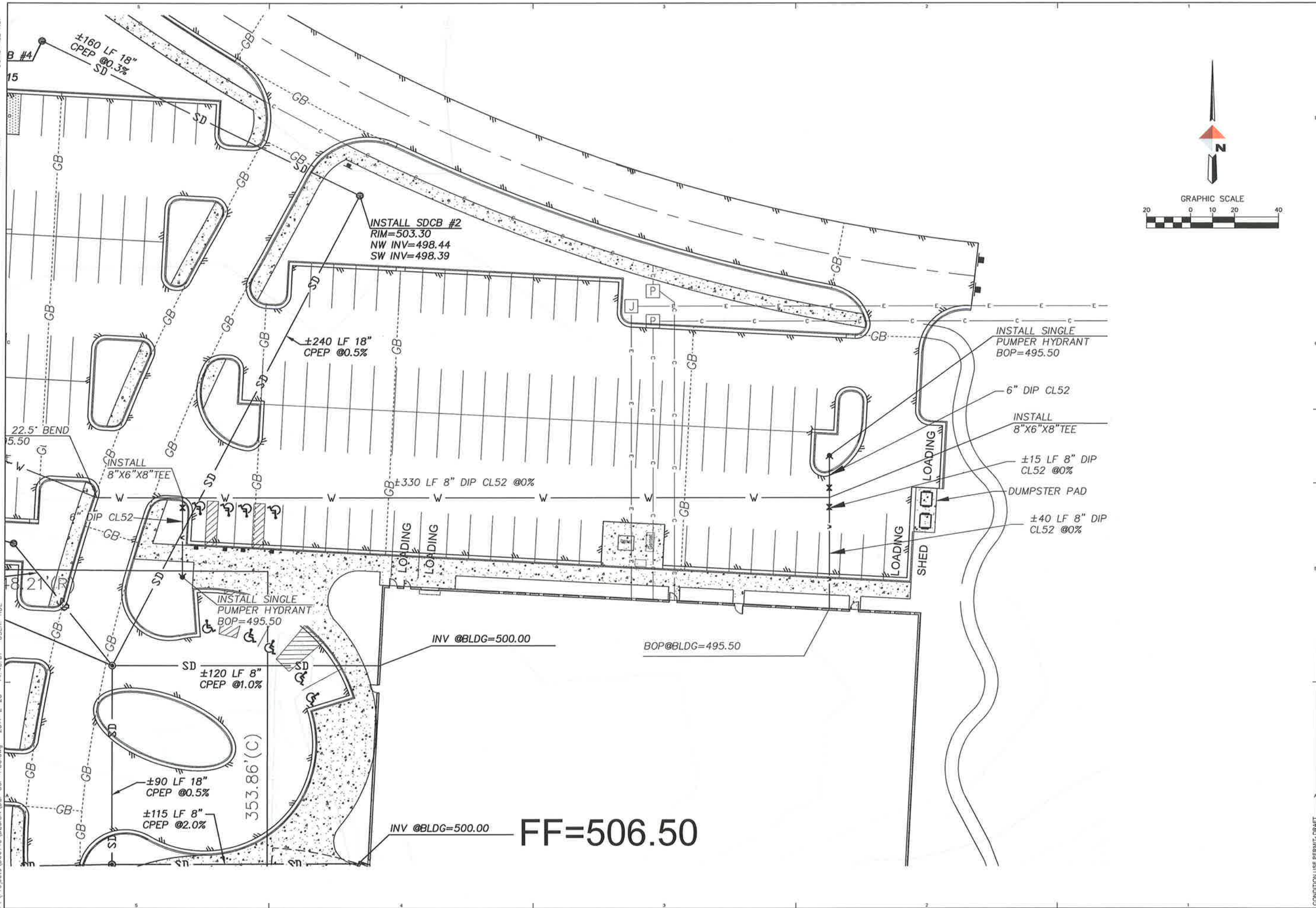
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JOB NO - kpb	A9061.01
JOB NO - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH
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SHEET NO C3.01	

CONDITION USE PERMIT DRAFT

FULL SIZE 22"X34" - HALF SIZE 11"X17"

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 nbbj
 575 14th Avenue, Suite 200, Anchorage, Alaska 99501
 (907) 255-5555 Fax: (907) 627-2300

NEESER CONSTRUCTION, INC.
 Anchorage, Alaska 99501
 Fax: (907) 276-8533
 2807 Blakely Road
 Office: (907) 276-1058

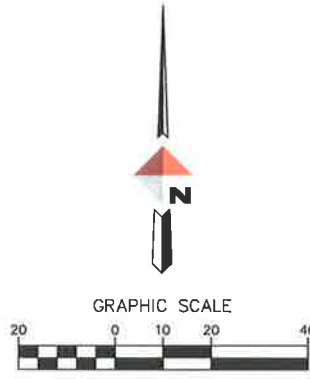


**SOUTH CENTRAL FOUNDATION
 NATIVE PRIMARY CARE CLINIC
 WASILLA, AK**

NO.	REVISION	SCHEDULE	DESCRIPTION	DATE

JOB NO. - HCI	11101
JOB NO. - kpl	A6081.01
JOB NO. - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH
SHEET NAME SITE GRADING PLAN	
SHEET NO. C3.02	

CONDITION: USE PERMIT - DRAFT
 FULL SIZE: 22" x 34" - HALF SIZE: 11" x 17"



CONNECT TO EXISTING
12" DIP WATER MAIN
BOP (12")=493.00
BOP (8")=493.33

EXISTING
RIM=503
NW INV=
SW INV=

KNIK GOOSE BAY ROAD

382.16'

N20°55'32"E

100'x60' Common Driveway Estm
Plat No. 2006+204

N89°37'38"W

INSTALL SDCB #5
NE INV=497.14

INSTALL SDMH #1
E INV=498.84
S INV=497.16
NE INV=497.26
NW INV=497.31

±75 LF 18"
CPEP @0.5%

INSTALL 22.5' BEND
BOP=495.50

INSTALL
8"x6"x8" TEE

INSTALL SDCB #3
RIM=502.50
NE INV=497.64

±75 LF 18"
CPEP @0.5%

INSTALL SINGLE
PUMPER HYDRANT
BOP=495.50

±120 LF 8"
CPEP @1.0%

±90 LF 18"
CPEP @0.5%

±115 LF 8"
CPEP @2.0%

INSTALL SDMH #2
N INV=496.73
S INV=496.68
E INV=497.74
W INV=496.78

±100 LF 18"
CPEP @0.5%

±170 LF 8" CPEP @2.0%

±240 LF 18"
CPEP @0.5%

353.86'(C)

0'06.3'E

100'x60'
Plat No.



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

REVISION	DESCRIPTION	DATE

JOB NO - NCI	1101
JOB NO - kp	A061.01
JOB NO - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
SITE GRADING PLAN

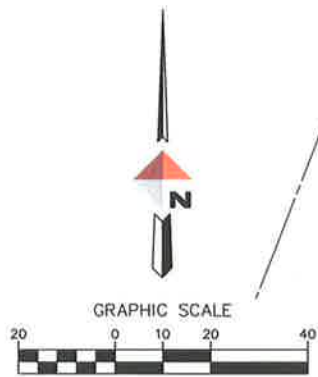
SHEET NO.
C3.03

CONDITION USE PERMIT - DRAFT
FULL SIZE 22"x34" 1/4"=1' SEE 11'x17'

P:\Projects\060715\DESIGN\DSI-SCF-PCC.dwg 2011-2-28 14:28:03 USER: RDL

SCRIPT FILE

DOWL FILE No



SOUTH KN

100'

382.16'

INV=494.68

SSMH
N Rim=493.58
Bottom=475.58
(full of water-unable to As-built)

INV=495.40

484.02'

Valve Box
Rim=494.80
Top of Nut=493.10

INV=495.07

±65 LF 12"
CPEP @0.50%

Valve Box
Rim=493.60
Top of Nut=484.00

24" CMP
N Inv=490.16
S Inv=489.55

EAST PALMER-WASILLA HIGHWAY

LOT
INSTALL SDCB #5
NE INV=497.14

±75 LF 18"
CPEP @0.5%

INSTALL
N INV=
S INV=
E INV=
W INV=

±100 LF 18"
CPEP @0.5%

±70 LF 18"
CPEP @0.5%

±170 LF 8"
SD

±170 LF 24" CPEP @0.50%
SD

±50 LF 24" CPEP @0.50%
Rim=495.94
Top of Nut=494.94

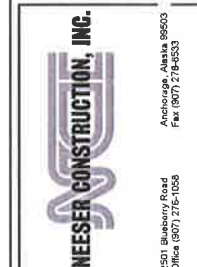
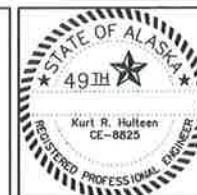
INSTALL STORMCEPTOR

Valve Box
Rim=495.38
Top of Nut=493.98

INSTALL SDCB
N INV=495.82
W INV=495.77

Valve Box
Rim=495.94
Top of Nut=494.04

S89°54'05"W



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

#	REVISION	DATE

JOB NO. - NCI	11101
JOB NO. - kpb	A0061.01
JOB NO. - nbbj	102748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
SITE GRADING PLAN

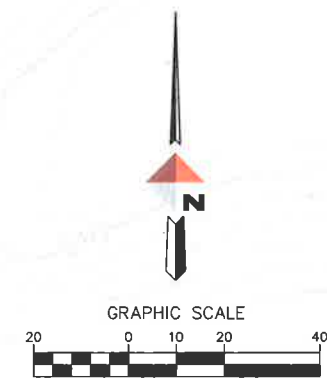
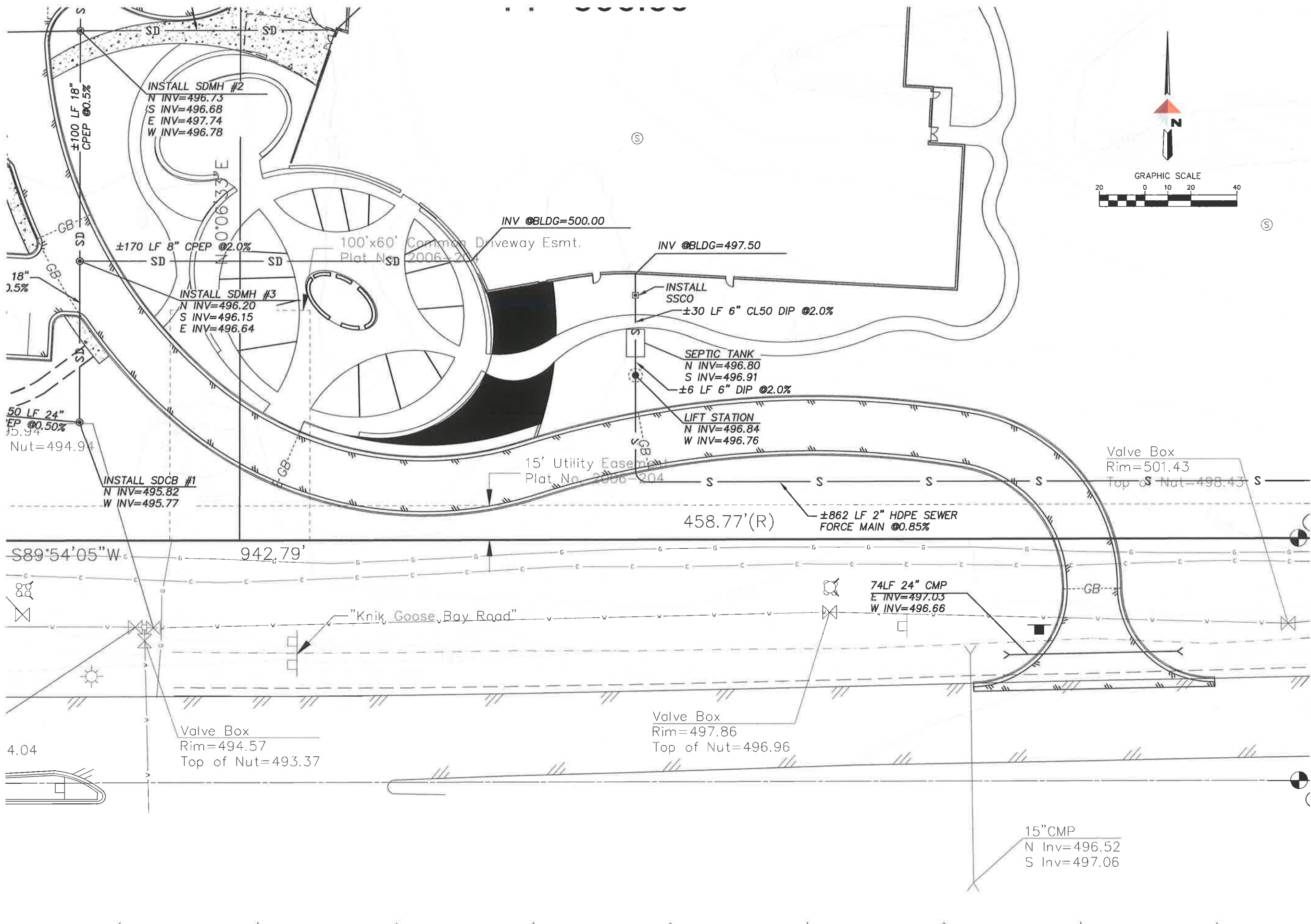
SHEET NO.
C3.04

CONDITION: USE PERMIT - DRAFT
FULL SIZE: 22" x 34" - HALF SIZE: 11" x 17"

DOWL FILE No:

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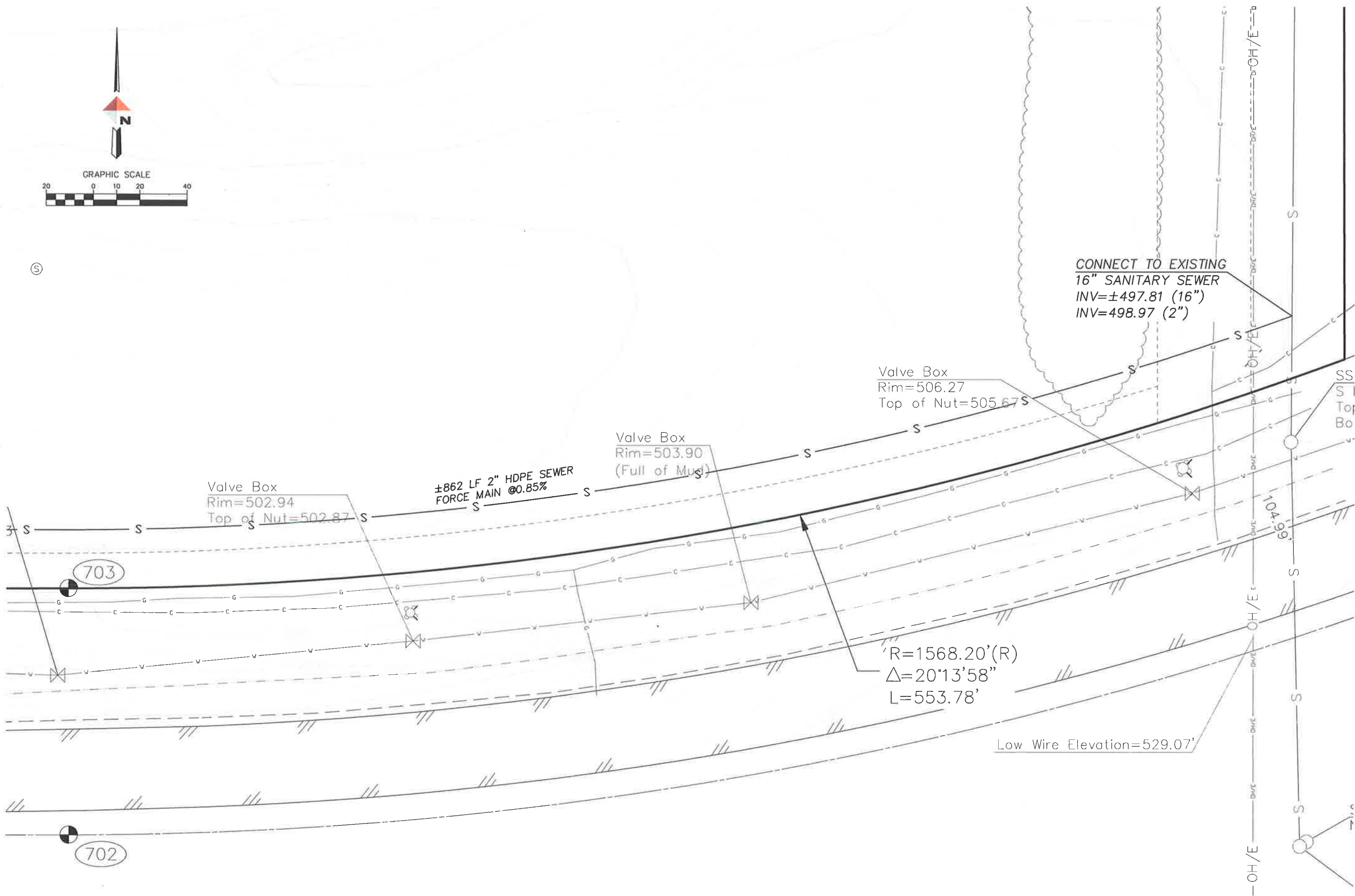
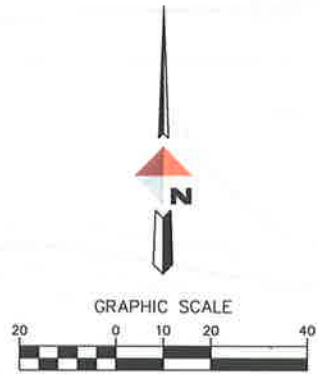
**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

REVISION	DESCRIPTION	DATE

JOB NO. - HCI	11101
JOB NO. - kpb	A9061.01
JOB NO. - nbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH
SHEET NAME SITE GRADING PLAN	
SHEET NO. C3.05	

CONDITION USE PERMIT DRAFT

FULL SIZE 22"x34" - HALF SIZE 11"x17"



CONNECT TO EXISTING
16" SANITARY SEWER
INV=±497.81 (16")
INV=498.97 (2")

Valve Box
Rim=506.27
Top of Nut=505.67

Valve Box
Rim=503.90
(Full of Mud)

Valve Box
Rim=502.94
Top of Nut=502.87

±862 LF 2" HDPE SEWER
FORCE MAIN @0.85%

'R=1568.20'(R)
Δ=20°13'58"
L=553.78'

Low Wire Elevation=529.07'



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

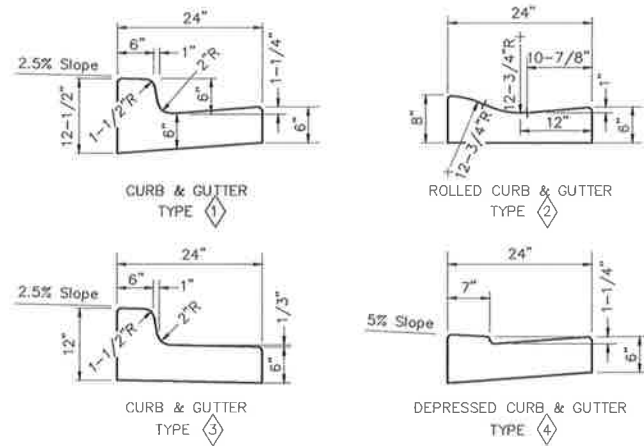
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JOB NO - kpb	A8061.01
JOB NO - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
SITE GRADING PLAN

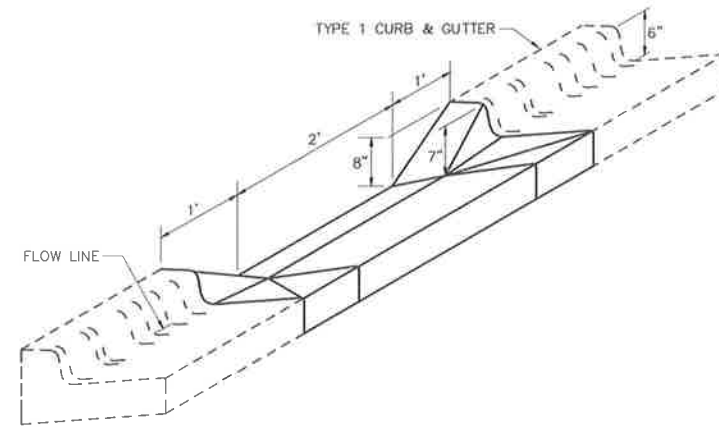
SHEET NO
C3.06

CONDITION: USE PERMIT, DRAFT
FULL SIZE: 24" x 36" HALF SIZE: 11" x 17"

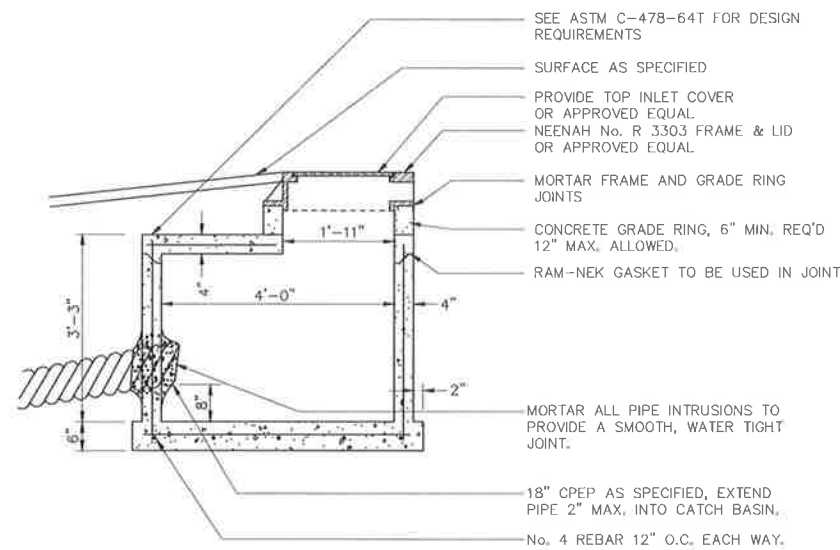


NOTE:
BOTH FRONT AND BACK EDGES OF THE CURB & GUTTER SHALL BE TROWELED TO A RADIUS OF ONE-HALF (1/2) INCH.

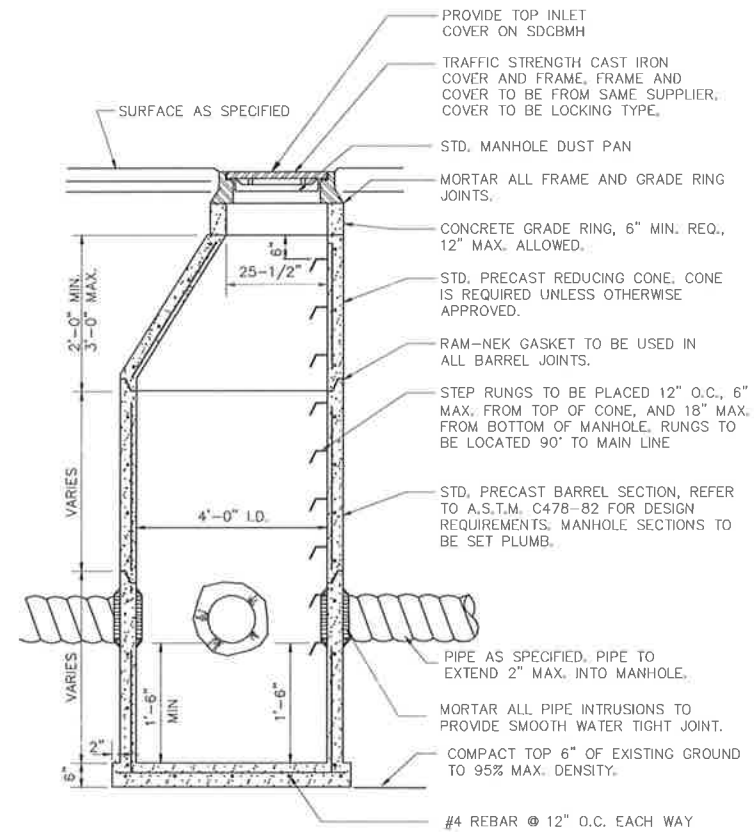
1 CURB DETAIL
C4.01 NTS



2 DRAINAGE CURB CUT
C4.01 NTS



3 STORM DRAIN CATCH BASIN
C4.01 NTS



4 STORM DRAIN DETAIL (TYPE 1)
C4.01 NTS



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

REVISION	SCHEDULE	DESCRIPTION	DATE
1			

JOB NO - HCI	11101
JOB NO - kpb	A9061.01
JOB NO - nbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
DETAILS

SHEET NO
C4.01

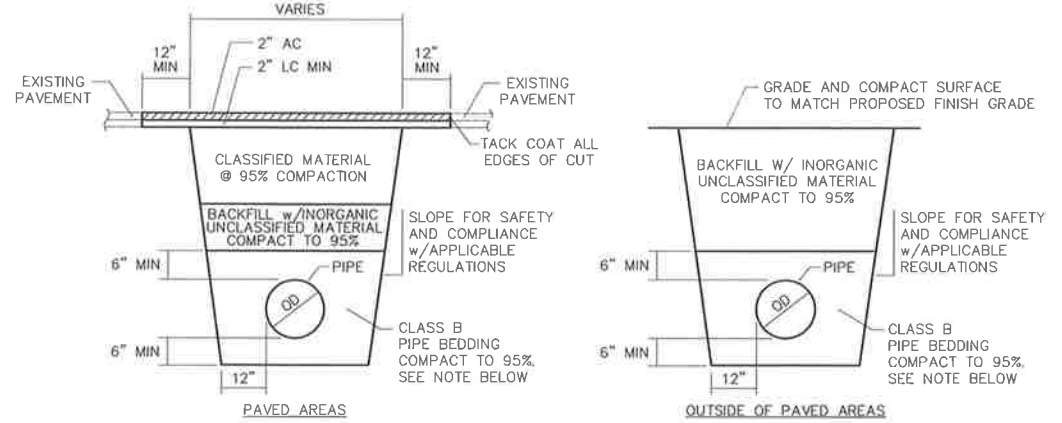
CONDITION: USE PERMIT, DRAFT

FULL SIZE: 24" x 36" HALF SIZE: 11" x 17"

DOML FILE NO

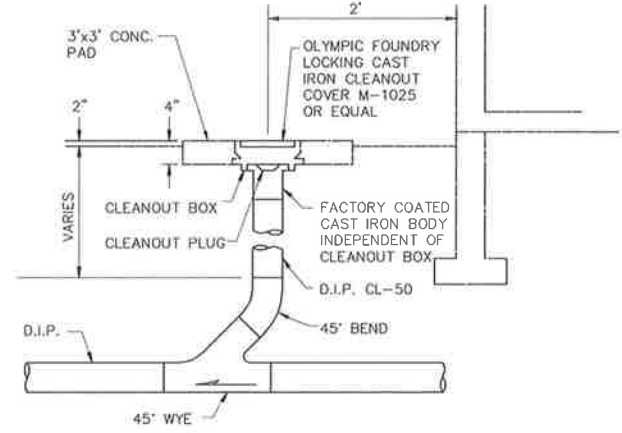
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- NOTES:
1. PIPE BEDDING TO SPRING LINE ONLY FOR ALL DUCTILE IRON PIPE.
 2. BEDDING MAY BE NATIVE MATERIAL APPROVED BY ENGINEER.

1
C4.02 TYPICAL UTILITY TRENCH SECTIONS
NTS



2
C4.02 SEWER SERVICE CLEANOUT
NTS



kpb architects
Kurt R. Hulteen
REGISTERED PROFESSIONAL ENGINEER
nbbj
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(907) 253-5555 Fax: (907) 617-1200

NEESER CONSTRUCTION, INC.
Anchorage, Alaska 99503
Fax: (907) 276-8533
2501 Blueberry Road
Office: (907) 276-1058

DOWL HKM
REGISTERED PROFESSIONAL ENGINEER
NO. 11202

**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

#	REVISION SCHEDULE	DESCRIPTION	DATE

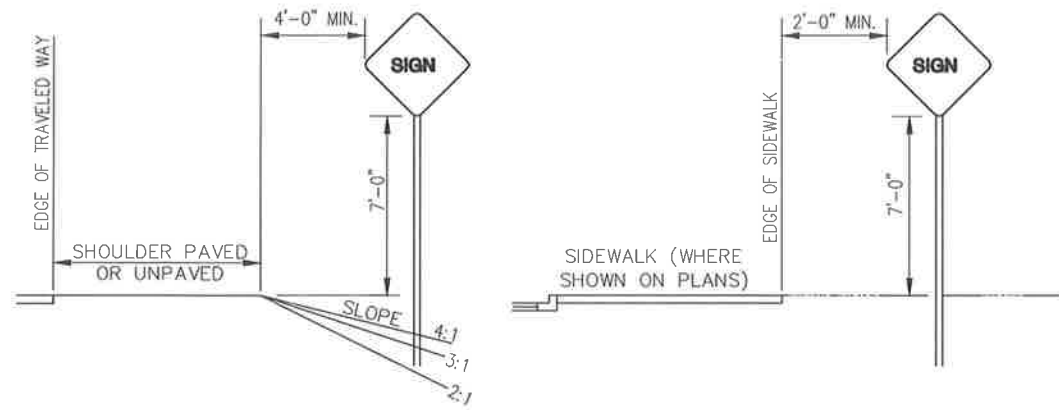
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JOB NO - kpb	A9061.01
JOB NO - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
DETAILS

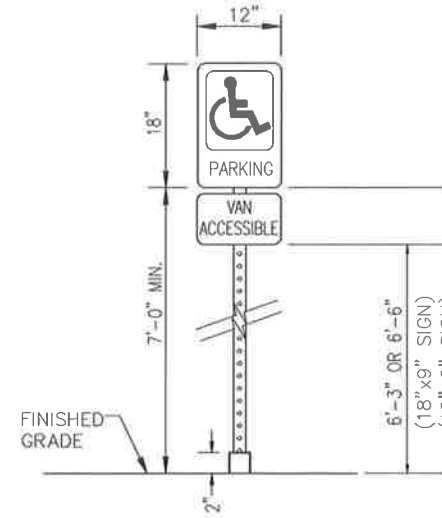
SHEET NO.
C4.02

CONDITION: USE PERMIT DRAFT

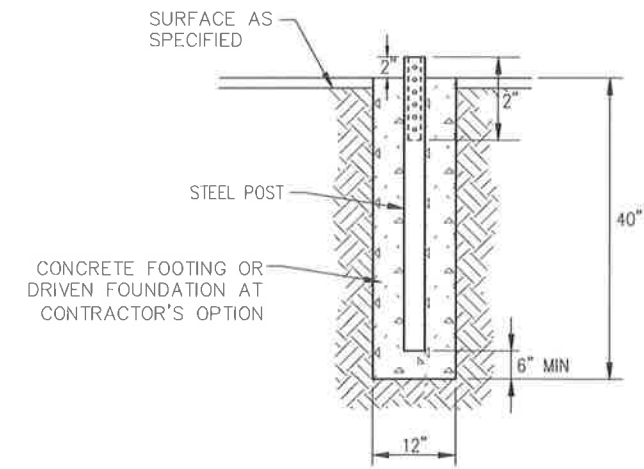
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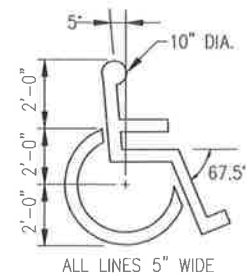
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C4.03 **SIGN LOCATION DETAIL**
NTS



2
C4.03 **HANDICAP PARKING SIGN DETAIL**
NTS

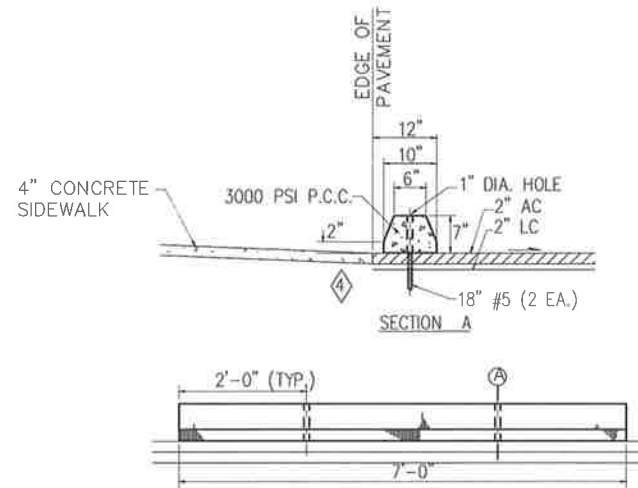


3
C4.03 **SIGN FOUNDATION DETAIL**
NTS



PROVIDE PAINTED WHEELCHAIR SYMBOL AT EACH DESIGNATED HANDICAP PARKING STALL. CENTER SYMBOL IN STALL.
ALL LINES 5" WIDE

4
C4.03 **WHEELCHAIR SYMBOL DETAIL**
NTS



5
C4.03 **BUMPER DETAIL (AT ALL HANDICAP PARKING STALLS)**
NTS



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

#	REVISION SCHEDULE	DESCRIPTION	DATE

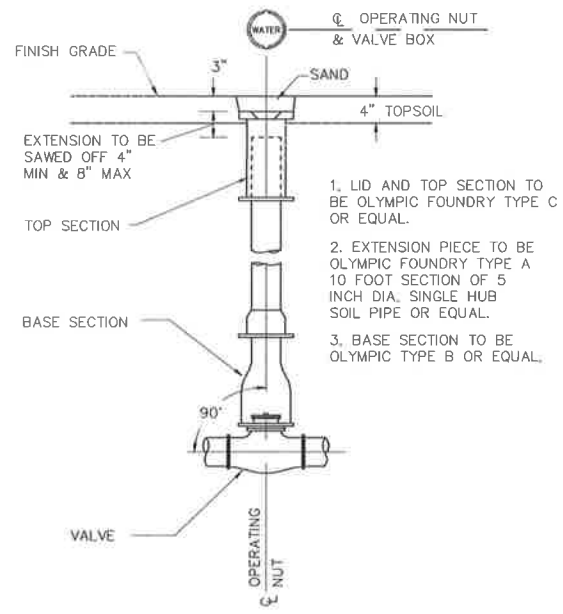
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JOB NO - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
DETAILS

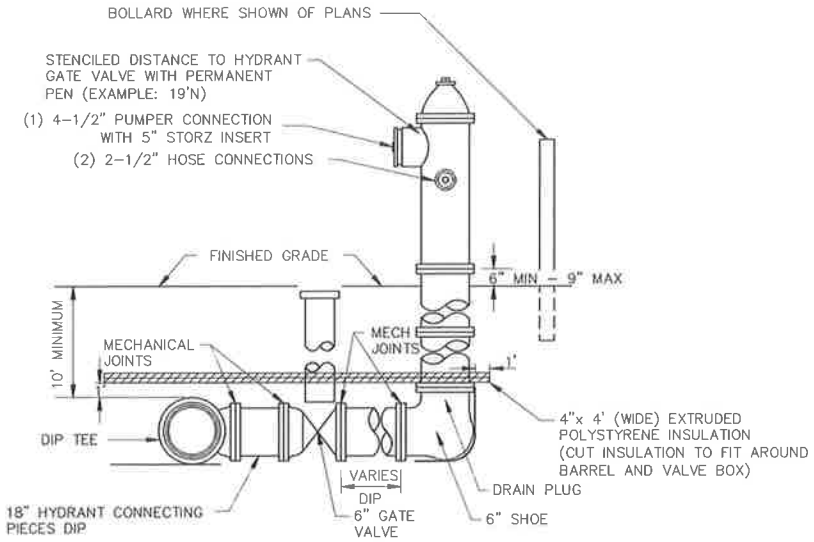
SHEET NO
C4.03

CONDITION USE PERMIT-DRAFT

FULL SIZE: 22"x34" - HALF SIZE: 11"x17"



1
C4.04 **VALVE BOX**
NTS



HYDRANT INSTALLATION NOTES:

1. HYDRANT BARREL MUST BE INSTALLED PLUMB AND THE LEG MUST BE INSTALLED LEVEL
2. DRAIN PLUG TO BE INSTALLED BY CONTRACTOR
3. TYPE IIA BACKFILL MATERIAL TO BE PLACED AROUND HYDRANT BARREL
4. ALL HYDRANTS SHALL BE PAINTED RED
5. AUXILIARY GATE VALVE BOX TO BE INSTALLED ACCORDING TO DETAIL FOR TYPICAL VALVE BOX
6. RESTRAIN ALL PIPE AND MECHANICAL JOINT FITTINGS FROM THE MAIN TO THE HYDRANT SHOE BY USE OF "MEG-A-LUG" AND/OR "FIELD-LOK" GASKETS
7. ALLOW NO BENDS IN HYDRANT LEG

2
C4.04 **SINGLE PUMPER HYDRANT ASSEMBLY**
NTS



**SOUTH CENTRAL FOUNDATION
NATIVE PRIMARY CARE CLINIC
WASILLA, AK**

REV	NO	SCHEDULE	DESCRIPTION	DATE

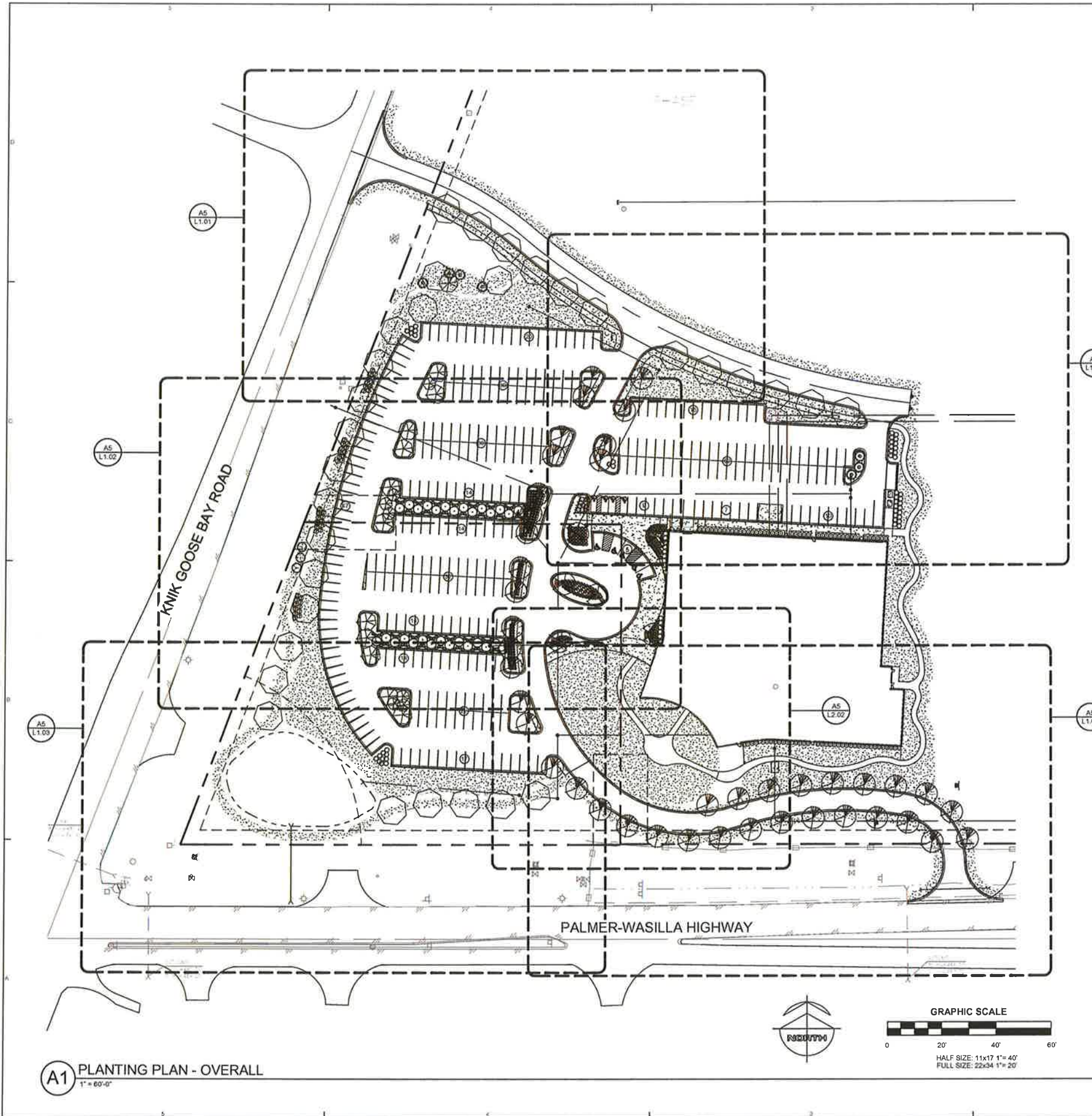
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JOB NO - kpb	A9061.01
JOB NO - nbbj	100748.00
DATE	03/01/2011
DRAWN	RDL
REVIEWED	KRH

SHEET NAME
DETAILS

SHEET NO.
C4.04

APPENDIX D

Landscape Plans



PLANT SCHEDULE						
SYMBOL	KEY	QTY	BOTANICAL NAME	COMMON NAME	SIZE	NOTES
TREES						
		32	BETULA POPYRIFERA	ALASKA PAPER BRCH	2.5' CAL	15' MAX HT
		6	PYRUS USSURIENSIS 'MORDAK'	PRAIRIE GEM FLOWERING PEAR	1.5' CAL	B&B
		87	QUERCUS MACROCARPA	BUR OAK	2.25' CAL	B&B
		7	BETULA POPYRIFERA	ALASKA PAPER BRCH - MULTI-STEM	1' - 2' CAL	B&B
		12	BETULA POPYRIFERA	ALASKA PAPER BRCH - MULTI-STEM	1.5' - 2.5' CAL	B&B
		7	LARIX SIBIRICA	SIBERIAN LARCH	6-7' HT	B&B
		31	PICEA PUNGENS	GREEN SPRUCE	7-8' HT	B&B
SHRUBS						
		51	PRUNUS TRILOBA 'MULTIPLY'	FLOWERING ALMOND	36" HT	#6 CONT.
		61	SYRINGA PATULA 'MISS KIM'	MISS KIM MANCHURIAN LILAC	36" HT	#6 CONT.
		73	SORBARIA SORBIFOLIA	URAL FALSE SPIREA	30" HT	#6 CONT.
		169	SPIREA STEVENII	ALASKA SPIREA	15" HT	#6 CONT.
		75	SPIREA x BUMALDA 'GOLDFLAME'	GOLDFLAME SPIREA	15" HT	#2 CONT.
		60	SPIREA x BUMALDA 'LITTLE PRINCESS'	LITTLE PRINCESS SPIREA	15" HT	#2 CONT.
		10	CORNUS SERICEA	RED-TWIG DOGWOOD	24" HT	#2 CONT.
		41	ROSA RUGOSA 'HANSA'	HANSA RUGOSA ROSE	3 CANES	#2 CONT. (MAGENTA)
		40	ROSA RUGOSA 'HENRY HUDSON'	HENRY HUDSON RUGOSA ROSE	3 CANES	#2 CONT. (WHITE)
		55	ROSA RUGOSA 'RUGELDA'	RUGELDA RUGOSA ROSE	3 CANES	#2 CONT. (YELLOW)

GENERAL NOTES

- CONTRACTOR SHALL NOTIFY LANDSCAPE ARCHITECT ABOUT SITE CONDITIONS THAT REQUIRE MODIFICATION OF PLANT LAYOUT PRIOR TO INSTALLATION OF AFFECTED LANDSCAPE MATERIAL
- CONTRACTOR SHALL RESTORE ALL DISTURBED AREAS WITH 4" TOPSOIL AND SEED.
- ALL PLANTING BEDS ADJACENT TO LAWN AREAS TO BE SEPARATED BY LANDSCAPE EDGING.

A1 PLANTING PLAN - OVERALL
1" = 60'-0"

STAMP

kpb architects

nbbj

NEESER CONSTRUCTION, INC.
 Anchorage, Alaska 99503
 2501 Muldrew Road
 Office (907) 274-1055
 Fax (907) 274-8533

CONSULTANT LOGO INFO

Southcentral Foundation
Valley Native Primary Care Center
Wasilla, Alaska

REVISION	DESCRIPTION	DATE

CONDITIONAL USE PERMIT APPLICATION

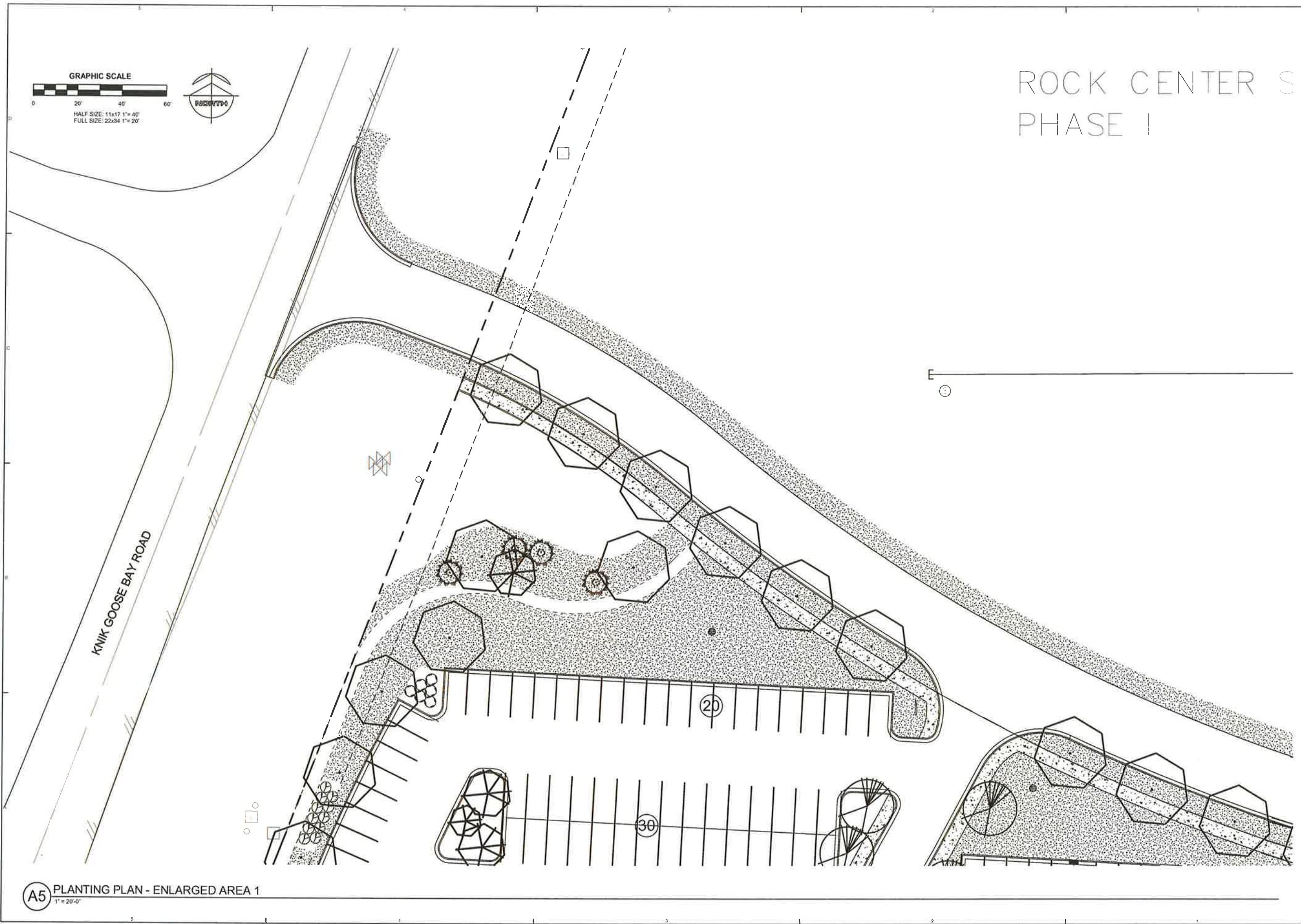
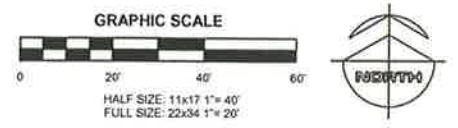
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JOB NO. - kpb	A9061.01
JOB NO. - nbbj	100748.00
DATE	03/01/2011
DRAWN	ERL
REVIEWED	Checker

SHEET NAME
PLANTING PLAN - OVERALL

SHEET NO.
L1.00

FULL SIZE: 22"x34" - HALF SIZE: 11"x17"

ROCK CENTER S PHASE I



A5 PLANTING PLAN - ENLARGED AREA 1
1" = 20'-0"

STAMP

kpb architects
nbbj

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2251 Blakely Road
Cordova, AK 99574
Phone: (907) 274-1258
Fax: (907) 274-6333

CONSULTANT LOGO / INFO

Southcentral Foundation
Valley Native Primary Care Center
Wasilla, Alaska

NO.	DESCRIPTION	DATE

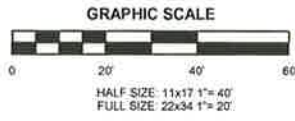
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JOB NO - nbbj	100749.00
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DRAWN	ERL
REVIEWED	Checker

SHEET NAME
PLANTING PLAN -
ENLARGED AREA 1

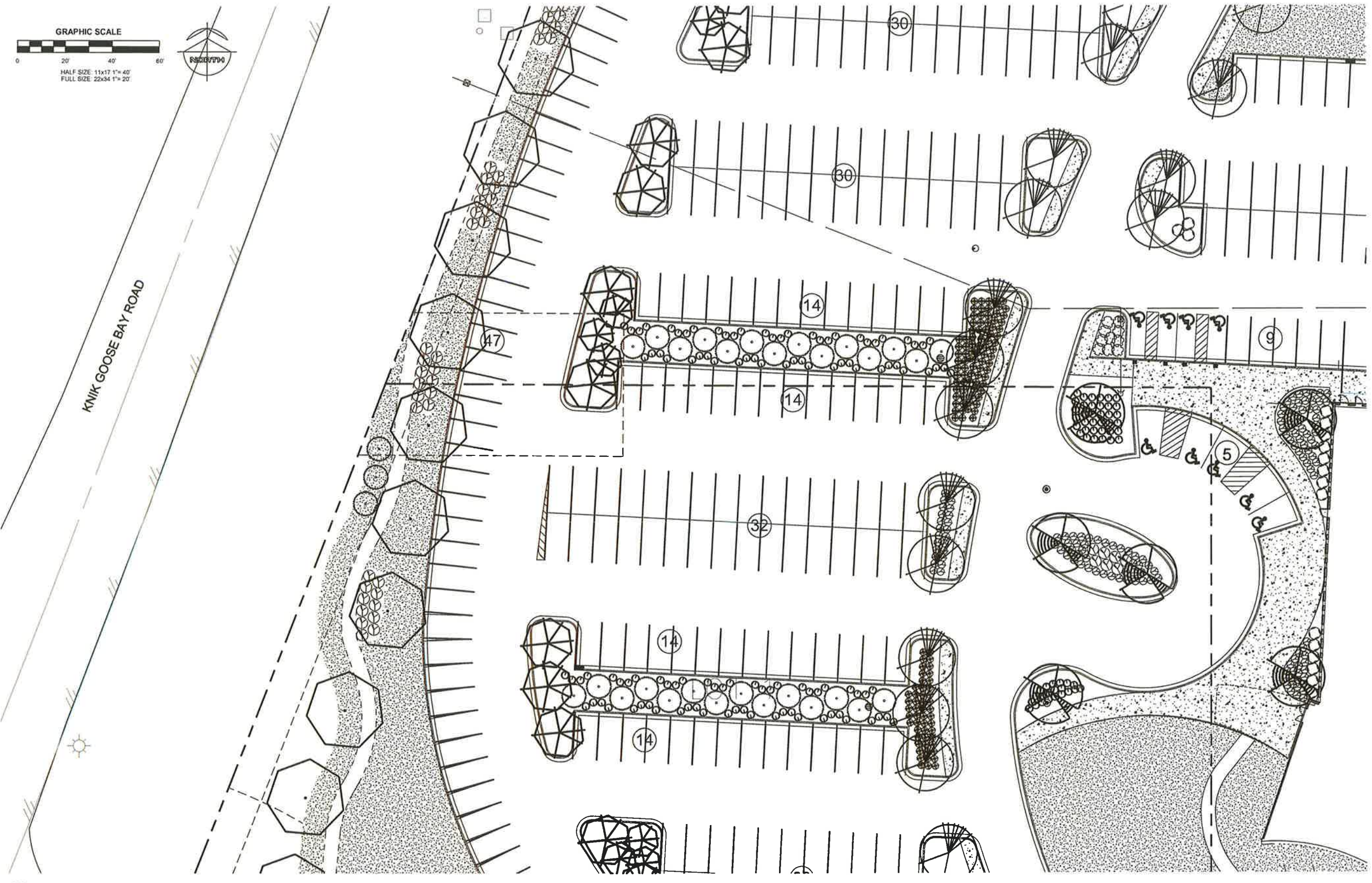
SHEET NO.
L1.01

CONDITIONAL USE PERMIT APPLICATION

FULL SIZE: 22x34" - HALF SIZE: 11x17"



KNIK GOOSE BAY ROAD



A5 PLANTING PLAN - ENLARGED AREA 2
1" = 20'-0"

STAMP

kpb architects
nbbj
1000 W. 11th Street, Suite 100
Wasilla, Alaska 99607
Phone: (907) 274-8333
Fax: (907) 274-8333

NEESER CONSTRUCTION, INC.
2501 Blakely Road
Chena (907) 276-1058
Anchorage, Alaska 99503
Fax: (907) 274-8333

CONSULTANT LOGO / INFO

Southcentral Foundation
Valley Native Primary Care Center
Wasilla, Alaska

NO.	REVISION/SCHEDULE	DATE

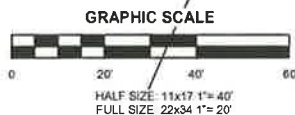
CONDITIONAL USE PERMIT APPLICATION

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JOB NO. - nbbj	100748.00
DATE	03/01/2011
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REVIEWED	Checker

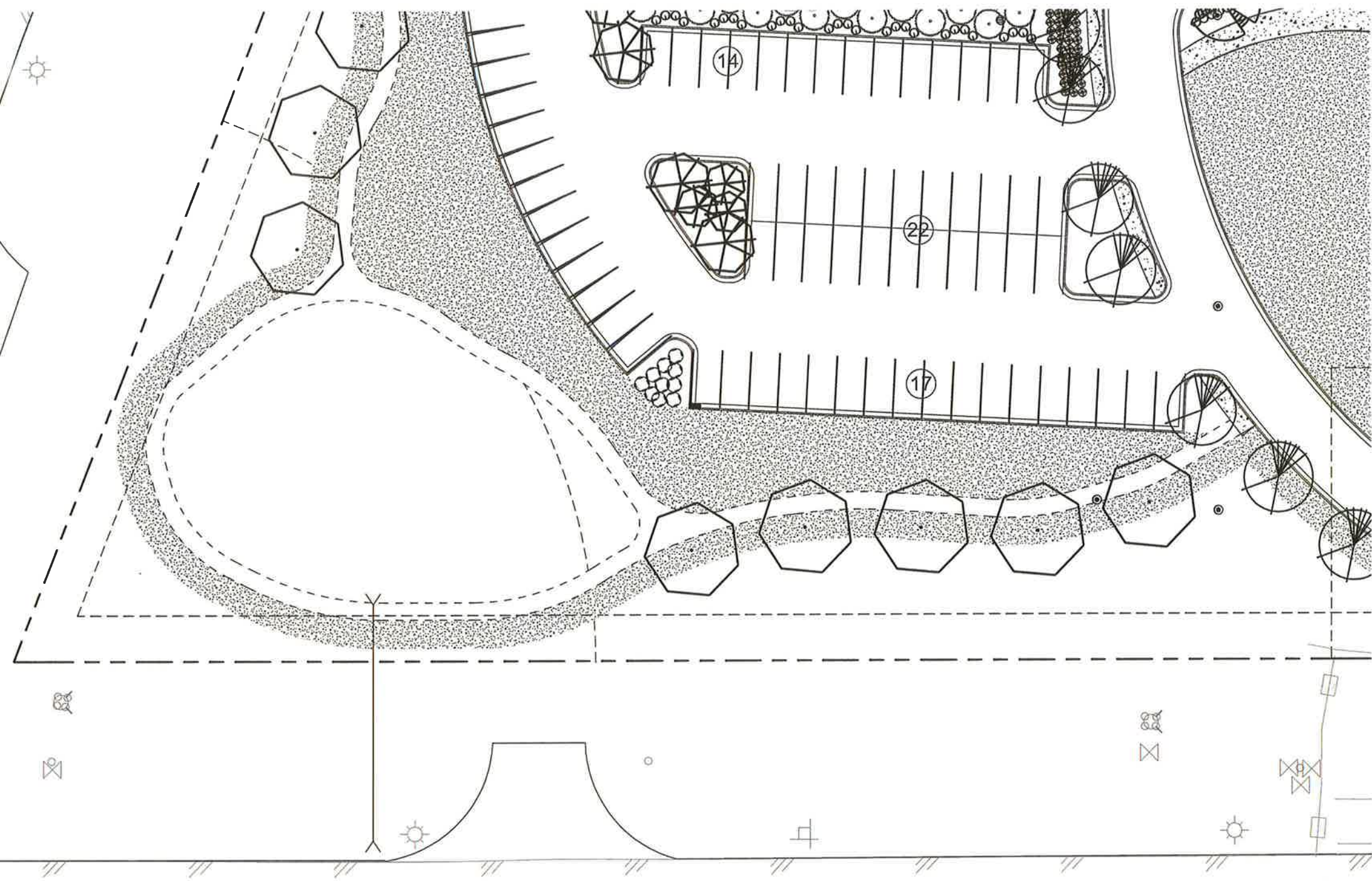
SHEET NAME
PLANTING PLAN - ENLARGED AREA 2

SHEET NO.
L1.02

FULL SIZE 22"x34" - HALF SIZE 11"x17"



KNIK GOOSE BAY ROAD



PALMER-WASILLA HIGHWAY

24" CVP
R=490.76

A5 PLANTING PLAN - ENLARGED AREA 3
1" = 20'-0"

STAMP

kp architects

nbbj

1201 W. ALASKA STREET, SUITE 100
ANCHORAGE, ALASKA 99501
PHONE: (907) 274-1058

NEESER CONSTRUCTION, INC.

1201 W. ALASKA STREET, SUITE 100
ANCHORAGE, ALASKA 99501
PHONE: (907) 274-1058

CONSULTANT LOGO / INFO

Southcentral Foundation
Valley Native Primary Care Center
Wasilla, Alaska

NO.	REVISIONS	DATE

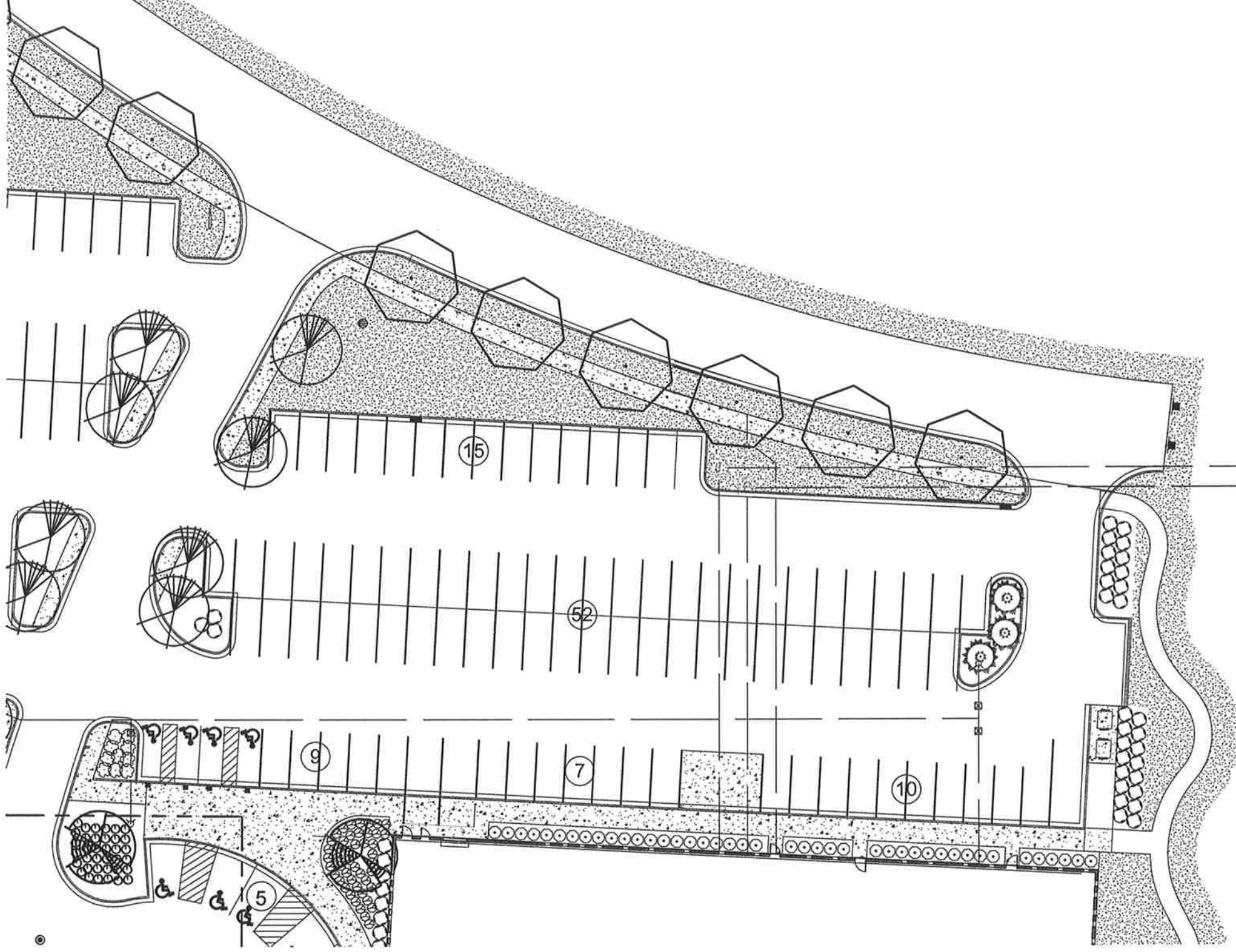
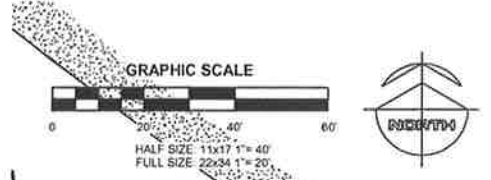
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JOB NO. - kps	A0001.01
JOB NO. - r44	100748.00
DATE	03/01/2011
DRAWN	ERL
REVIEWED	Checker

SHEET NAME
PLANTING PLAN - ENLARGED AREA 3

SHEET NO.
L1.03

CONDITIONAL USE PERMIT APPLICATION

FULL SIZE: 24"X36" - HALF SIZE: 11"X17"



A5 PLANTING PLAN - ENLARGED AREA 4
1" = 20'-0"

STAMP

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nbbj
1000 7th Avenue, Suite 1000, Wasilla, Alaska 99587
Tel: (907) 276-8333 Fax: (907) 276-8333

NEESER CONSTRUCTION, INC.
2501 Blueberry Road
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Fax (907) 276-8533

CONSULTANT LOGO / INFO

**Southcentral Foundation
Valley Native Primary Care Center
Wasilla, Alaska**

NO.	REVISION / SCHEDULE	DATE

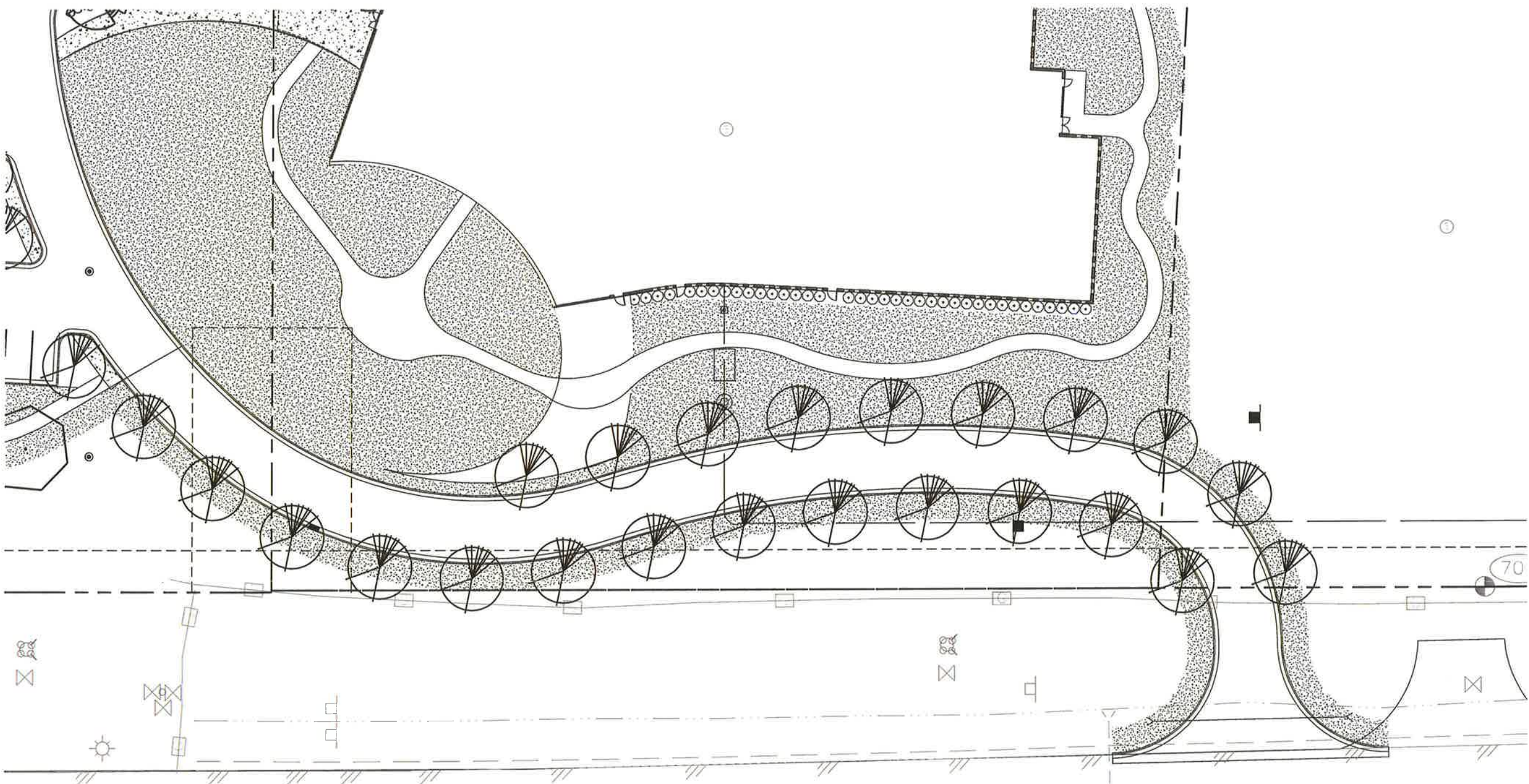
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JOB NO. - kpb	A0061.01
JOB NO. - nbbj	100748.00
DATE	03/01/2011
DRAWN	ERL
REVIEWED	Checker

SHEET NAME
PLANTING PLAN - ENLARGED AREA 4

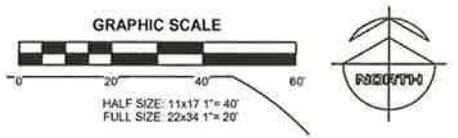
SHEET NO.
L1.04

CONDITIONAL USE PERMIT APPLICATION

FULL SIZE: 22x34" - HALF SIZE: 11x17"



PALMER-WASILLA HIGHWAY



A5 PLANTING PLAN - ENLARGED AREA 5
1" = 20'-0"

15' CV =
N = 496.52
S = 497.06

STAMP

kpb architects
nbbj
1200 W. 10th Avenue, Suite 100
Wasilla, Alaska 99607
Tel: (907) 276-8533

NEESEY CONSTRUCTION, INC.
2501 Markham Road
Office (907) 276-1005
Mechanics, Alaska #0600
Fire (907) 276-8533

CONSULTANT LOGO INFO

Southcentral Foundation
Valley Native Primary Care Center
Wasilla, Alaska

NO.	REVISION/CHANGE DESCRIPTION	DATE

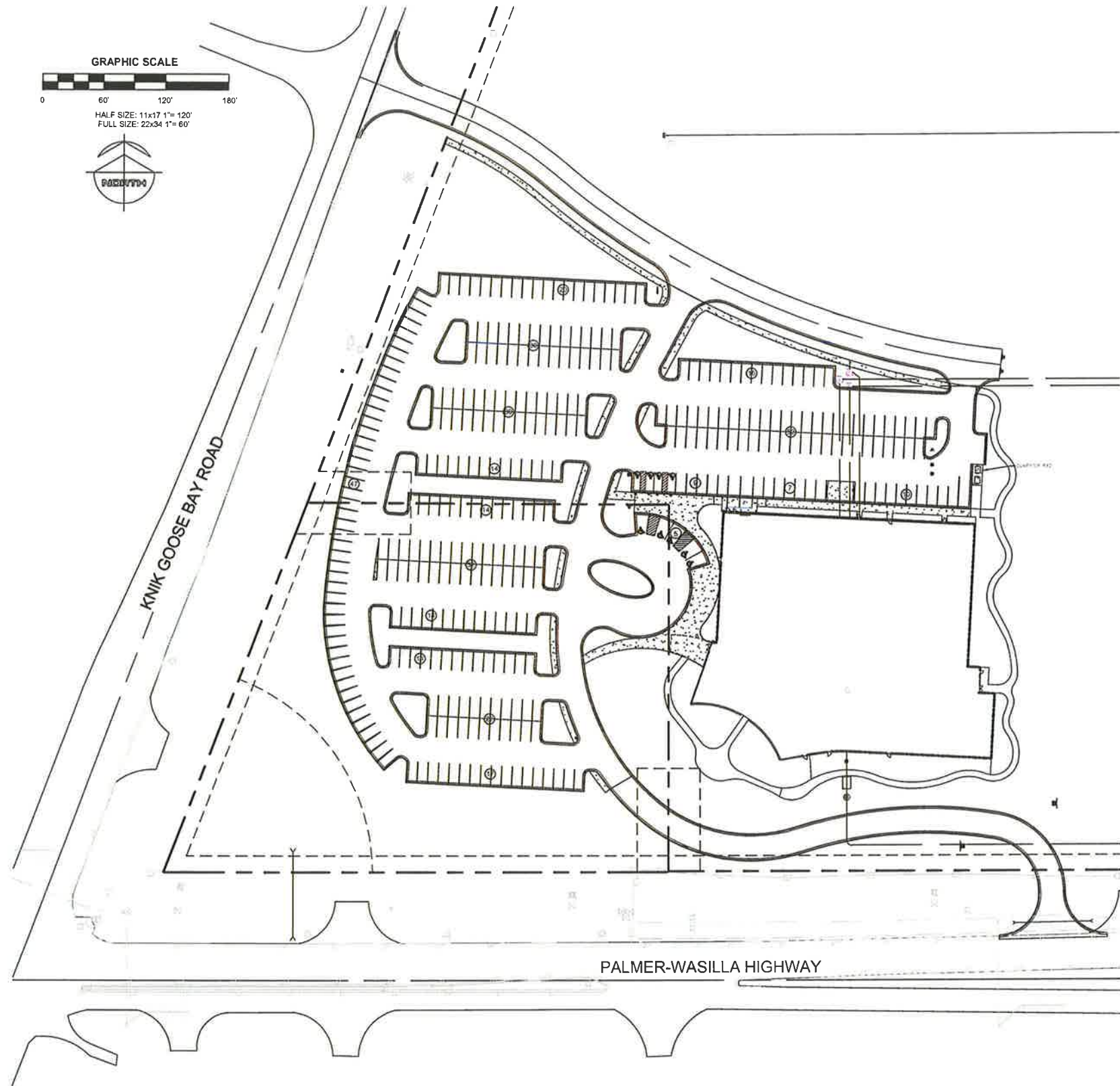
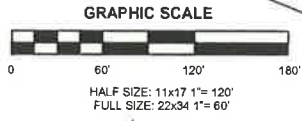
JOB NO. - NCI 11101
JOB NO. - kpb A9061.01
JOB NO. - nbbj 100748.00
DATE 03/01/2011
DRAWN ERL
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SHEET NAME
PLANTING PLAN - ENLARGED AREA 5

SHEET NO.
L1.05

CONDITIONAL USE PERMIT APPLICATION

FULL SIZE 22"x34" - HALF SIZE 11"x17"



1 SITE PLAN - BASE BID
1" = 60'-0"

STAMP

kpb architects

nbbj

1000 W. 10th Ave., Suite 1000
Anchorage, Alaska 99501
Phone: (907) 276-4533
Fax: (907) 276-4533

NEESER CONSTRUCTION, INC.

2501 Blakely Road
Anchorage, Alaska 99503
Office: (907) 276-1955

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Wasilla, Alaska**

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JOB NO. - nbbj	100748.00
DATE	03/01/2011
DRAWN	Author
REVIEWED	Checker

SHEET NAME
SITE PLAN - BASE BID

SHEET NO.
L2.00

CONDITIONAL USE PERMIT APPLICATION

FULL SIZE: 22x34" - HALF SIZE: 11x17"

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kp architects

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1000 S. W. 10th Ave., Suite 1000
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Fax: (503) 227-1101

NEESER CONSTRUCTION, INC.

2501 Blakely Road
Cordova, Alaska 99574
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Fax: (907) 276-8333

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NOT LOGO /
INFO

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Valley Native Primary Care Center
Wasilla, Alaska

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JOB NO - A26	A0051.01
JOB NO - 668	100746.00
DATE	03/01/2011
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REVIEWED	Checker

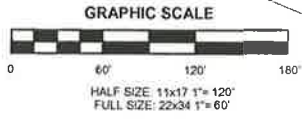
SHEET NAME
SITE PLAN - BETTERMENTS

SHEET NO.
L2.01

CONDITIONAL USE PERMIT APPLICATION

FULL SIZE: 22x34" - HALF SIZE: 11x17"

PHASE 1



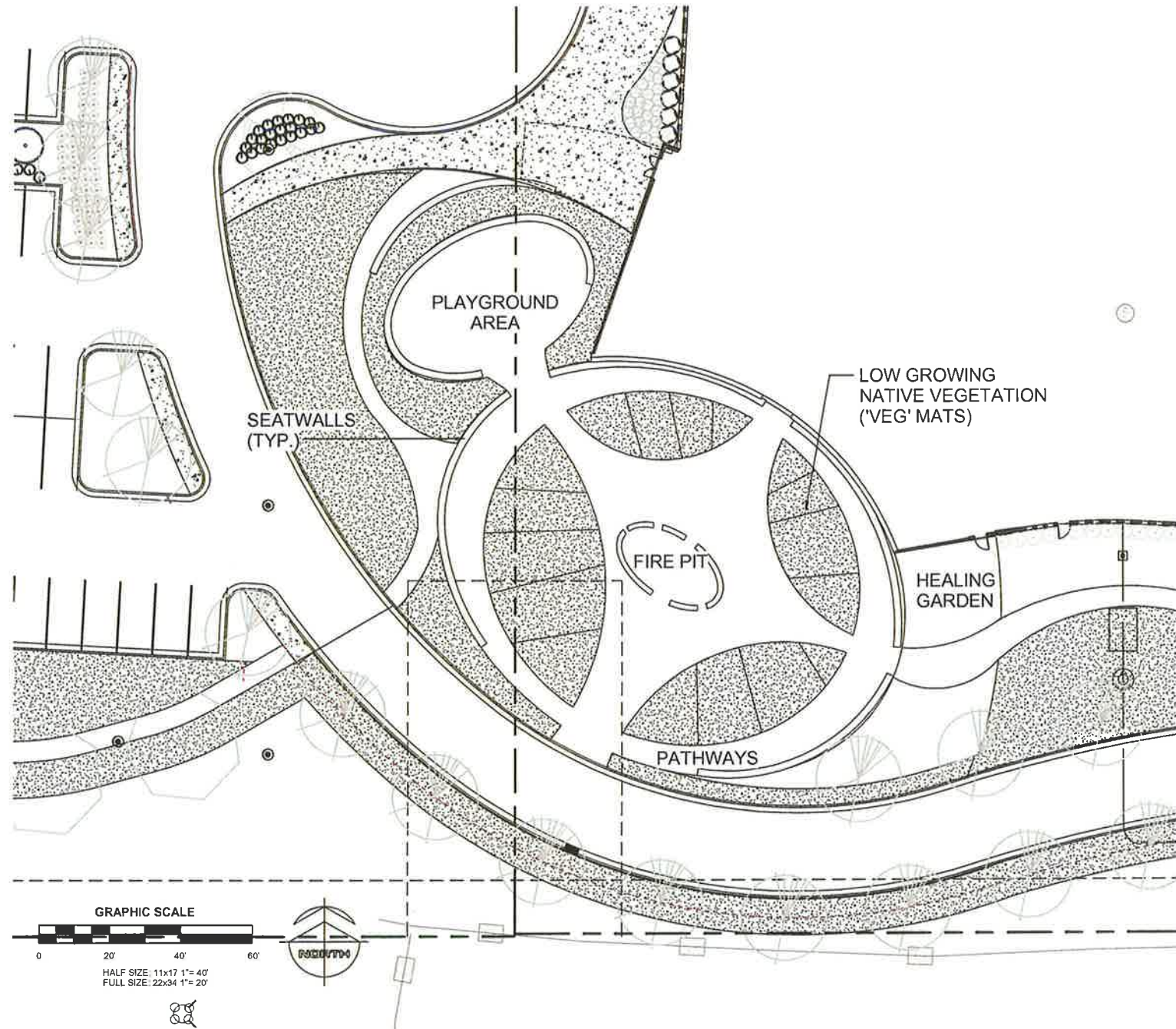
GRAVEL PATHWAY
BETTERMENT

KNIK GOOSE BAY ROAD

PLAYGROUND & GATHERING
AREA BETTERMENT

PALMER-WASILLA HIGHWAY

1 SITE PLAN - BETTERMENTS
1"=60'-0"



A5 LANDSCAPE BETTERMENT
1" = 20'-0"

STAMP

kpb architects

nbbj

225 South Main Street, Suite 200
Wasilla, Alaska 99587
Phone: (907) 274-2200
Fax: (907) 274-2202

NEESER CONSTRUCTION, INC.

201 Blakely Road
Office (907) 278-1006

Anchorage, Alaska 99503
Fax (907) 278-4533

CONSULTANT LOGO / INFO

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Valley Native Primary Care Center
Wasilla, Alaska

REVISED	DESCRIPTION	DATE

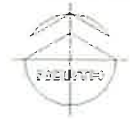
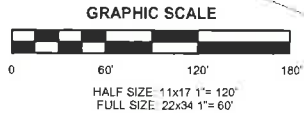
CONDITIONAL USE PERMIT APPLICATION

JOB NO. - NCS 11101
JOB NO. - kpb A9061.01
JOB NO. - nbbj 100748.00
DATE 03/01/2011
DRAWN ERL
REVIEWED Checker

SHEET NAME
PLANTING PLAN -
BETTERMENTS

SHEET NO.
L2.02

FULL SIZE: 22x34 - HALF SIZE: 11x17



PHASE 1



15% INTERIOR LANDSCAPING CALCULATION:

TOTAL AREA CONSISTING OF PAVED PARKING AND DRIVELANES: 154,477 s.f.
CALCULATION: 154,477 x 0.15 = 23,171 s.f.
TOTAL AREA CONSISTING OF PARKING LOT LANDSCAPE: 24,508 s.f.

SNOW STORAGE CALCULATION:

TOTAL NUMBER OF PARKING SPACES: 352
CALCULATION: 352 x 25s.f. = 8,800 s.f.
TOTAL AREA OF SNOW STORAGE SHOWN: 12,800 s.f.

LEGEND

- SNOW STORAGE
- INTERIOR LANDSCAPE
- PERIMETER LANDSCAPE

STAMP

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225 W. 10th Street, Suite 1000, Anchorage, Alaska 99501
Office: (907) 276-1000 Fax: (907) 276-9533

NEESER CONSTRUCTION, INC.

2501 Blueberry Road, Anchorage, Alaska 99503
Office: (907) 276-1000 Fax: (907) 276-9533

CONSULTANT LOGO / INFO

Southcentral Foundation
Valley Native Primary Care Center
Wasilla, Alaska

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JOB NO - kps	A0001.01
JOB NO - nbbj	100748.00
DATE	03/01/2011
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SHEET NAME
SITE PLAN - Snow Storage, Interior Landscape & Perimeter Landscape

SHEET NO.
L2.03

1 INTERIOR LANDSCAPE/SNOW STORAGE AREA CALCULATIONS
1" = 60'-0"

CONDITIONAL USE PERMIT APPLICATION

FULL SIZE: 22x34" - HALF SIZE: 11x17"

APPENDIX E

Lighting Cut Sheets



FEATURES & SPECIFICATIONS

INTENDED USE — Streets, walkways, parking lots and surrounding areas.

CONSTRUCTION — Rugged, die-cast, single-piece aluminum housing with nominal wall thickness of 1/8". Die-cast door frame has impact-resistant, tempered, glass lens (3/16" thick). Door frame is fully gasketed with one-piece tubular silicone. **US. Patent No. D447,590. Canada Patent No. 94324.**

FINISH — Standard finish is dark bronze polyester powder finish. Additional architectural colors are available; see www.lithonia.com/archcolors.

OPTICAL SYSTEM — Anodized segmented reflectors for superior uniformity and control. Reflectors attach with tool-less fasteners and are rotatable and interchangeable. Five full cutoff distributions available: Type II (roadway), Type III (asymmetric), Type IV (forward throw), Type IV (wide, forward throw) and Type V (symmetric square).

ELECTRICAL SYSTEM — Ballast: Constant-wattage autotransformer ballast standard. Super CWA pulse-start ballast required 200W, 320W and 350W (SCWA option). Ballast is copper-wound and 100% factory-tested. All ballasts are mounted on a removable power tray with tool-less latch and have positive locking disconnect plugs. Super CWA Pulse Start ballasts, 88% efficient and EISA legislation compliant, are required for 200-400W (must order SCWA option) for US shipments only. CSA, NOM or INTL required for probe start shipments outside of the US.

Socket: Porcelain, horizontally-mounted, mogul-base socket with copper alloy, nickel-plated screw shell and center contact.

INSTALLATION — Integral arm for pole or wall mounting. Optional mountings available.

LISTING — UL Listed (standard). CSA Certified (see Options). UL listed for 25°C ambient and wet locations. IP65 Rated.

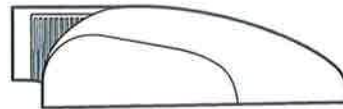
NOTE: Specifications subject to change without notice.

Catalog Number	
Notes	Type

AERIS™

Architectural Area & Roadway Luminaires

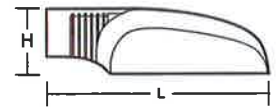
AS2



METAL HALIDE: 200W-400W
HIGH PRESSURE SODIUM: 200W-400W
 10' to 35' Mounting

Specifications

EPA: 1.2 ft²
 Length: 28.6 (72.6)
 Width: 17.1 (43.4)
 Depth: 8.3 (21.0)
 *Weight: 40 lbs (18.2 kg)
 *Weight as configured in example below.



All dimensions are inches (centimeters) unless otherwise specified.

ORDERING INFORMATION

For shortest lead times, configure product using **standard options (shown in bold)**.

Example: AS2 250M SR3 TB SCWA SPA LPI

AS2	400S	SR4W	480			QRS TD ASW2VG W/LU	DSPJ	LPI
Series	Wattage ¹		Voltage	Mounting	Ballast	Options	Finish ¹³	Lamp ¹⁴
AS2	Metal halide		120	SPA Square pole mounting	(blank) Magnetic ballast	Shipped installed in fixture	(blank) Dark bronze	LPI Lamp in-chubed
	200M ²		208 ⁵	RPA Round pole mounting	CWI Constant wattage isolated	SF Single fuse 120, 277, 347V	DBL Black	L/LP Less lamp
	250M		240 ⁶	WBA Wall bracket (up or down) ⁹	Pulse Start	DF Double fuse 208, 240, 480V	DGC Charcoal gray	
	320M ²		277	ASKMA2 Mast arm adapter ¹⁰	E	PER NEMA twist-lock receptacle only (no photocontrol)	DMB Medium bronze	
	350M ^{2,3}		347	DCAS2 Decorative curved arm, square pole only ¹⁰	SCWA Super CWA pulse start ballast	EC Emergency circuit ¹¹	DNA Natural aluminum	
	400M ²		480 ⁶	DCAS2R Decorative curved arm, round pole only ¹⁰	NOTE: For shipments to U.S. territories, SCWA must be specified to comply with EISA.	HS Houseside shield (SR2, SR3, SR4W)	DWH White	
	High pressure sodium ⁴		TB ⁴			CSA CSA Certified	CR Corrosion resistance	
	200S		23050HZ ⁹			NOM NOM Certified		
	250S					INTL Available for MH probe start shipping outside the U.S.		
	400S					Shipped separately ¹²	DSPJ-grey	
						PE1 NEMA twist-lock PE (120, 208, 240V)		
						PE3 NEMA twist-lock PE (347V)		
						PE4 NEMA twist-lock PE (480V)		
						PE7 NEMA twist-lock PE (277V)		
						VG Vandal guard		
						SC Shorting cap		

Distribution
SR2 Segmented type II roadway
SR3 Segmented type III asymmetric
SR4SC Segmented type IV forward throw, sharp cutoff
SR4W Segmented type IV wide, forward throw
SR5S Segmented type V square

- NOTES:
- Lower wattages available. Consult factory.
 - Must be ordered with SCWA.
 - Must use reduced jacket lamp.
 - Not available with SCWA.
 - Must specify CWI for use in Canada.
 - Optional multi-tap ballast (120, 208, 240, 277V) (120, 277, 347V in Canada).
 - Consult factory for available wattage.⁹
 - Mounted in lens-up orientation, fixture is damp location listed.
 - Shipped separately.
 - Maximum allowance wattage lamp included.
 - May be ordered as an accessory.
 - Additional architectural colors available; see www.lithonia.com/archcolors.
 - Must be specified.

Note: Aeris™ has a unique drilling template that requires an Aeris drilling pattern to be specified when ordering poles. See example below.
 Example: SSA 20 4C DM19AS DDB

Aeris Drilling Pattern

DM19AS	1 at 90 degrees
DM28AS	2 at 180 degrees
DM28AS	2 at 90 degrees
DM38AS	3 at 90 degrees
DM48AS	4 at 90 degrees
DM32AS	3 at 120 degrees (round poles only)

ASW2VG-wire guard



Accessories: Tenon Mounting Slipfitter

Order as separate catalog number. Must be used with round pole mounting (RPA).

Tenon O.D.	One	Two@180°	Two@90°	Three@120°	Three@90°	Four@90°
2-3/8"	AST20-190	AST20-280	AST20-290	AST20-320	AST20-390	AST20-490
2-7/8"	AST25-190	AST25-280	AST25-290	AST25-320	AST25-390	AST25-490
4"	AST35-190	AST35-280	AST35-290	AST35-320	AST35-390	AST35-490

AS2VG Vandal guard

APPENDIX F

Signage and Wayfinding



EXTERIOR IDENTIFICATION AND WAYFINDING SIGNAGE



BUILDING IDENTIFICATION SIGNAGE - WEST ELEVATION



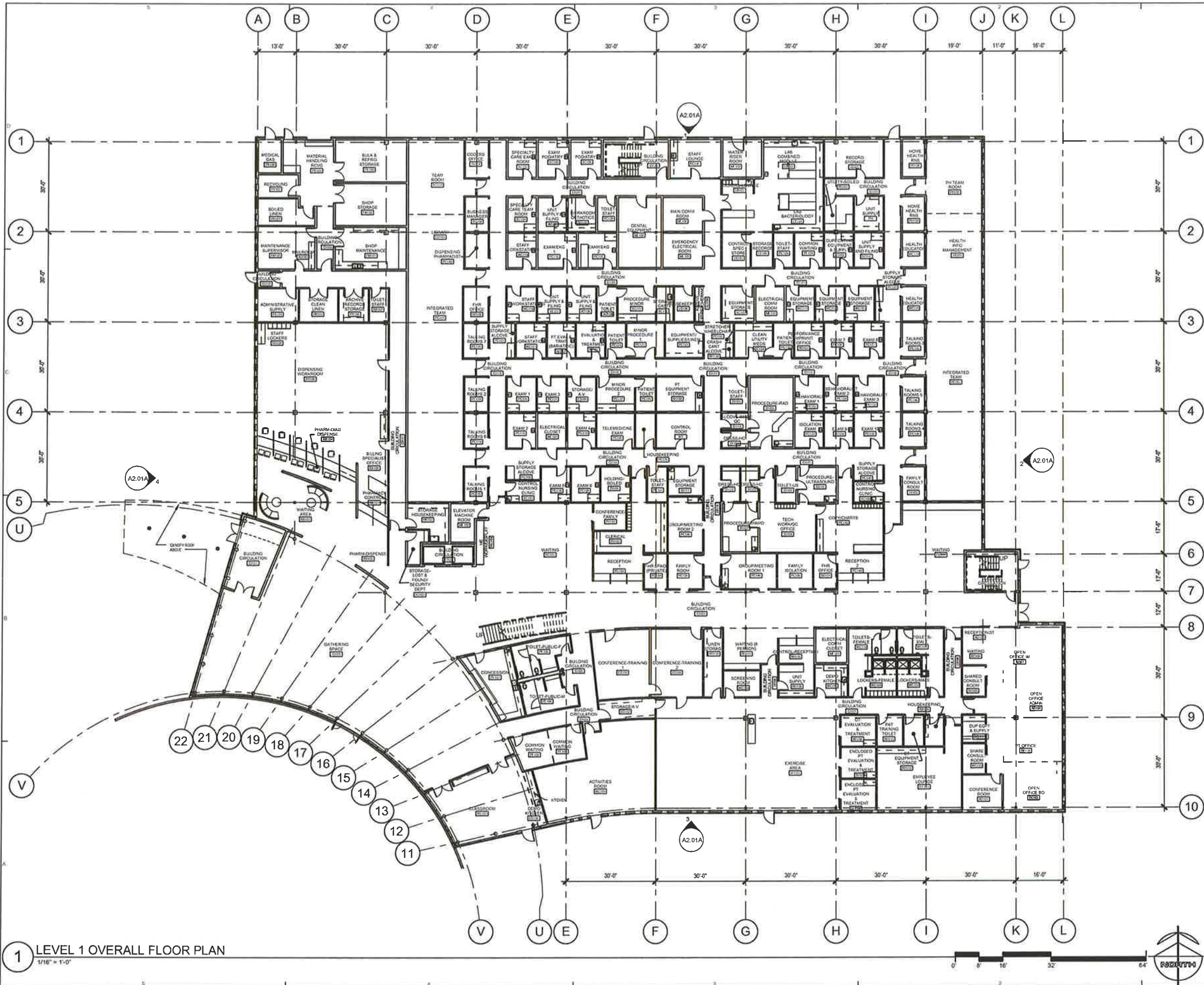
INTERIOR IDENTIFICATION AND WAYFINDING SIGNAGE



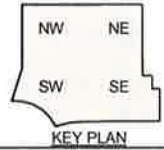
SITE IDENTIFICATION AND WAYFINDING SIGNAGE

APPENDIX G

Architectural Plans



1 LEVEL 1 OVERALL FLOOR PLAN
1/16" = 1'-0"



KEY PLAN

STAMP

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 mbbj
2501 Markway Road
Anchorage, Alaska 99503
Office (907) 274-8333

NEESER CONSTRUCTION, INC.
2501 Markway Road
Anchorage, Alaska 99503
Office (907) 274-8333

CONSULTA
NOT LOGG
INFO

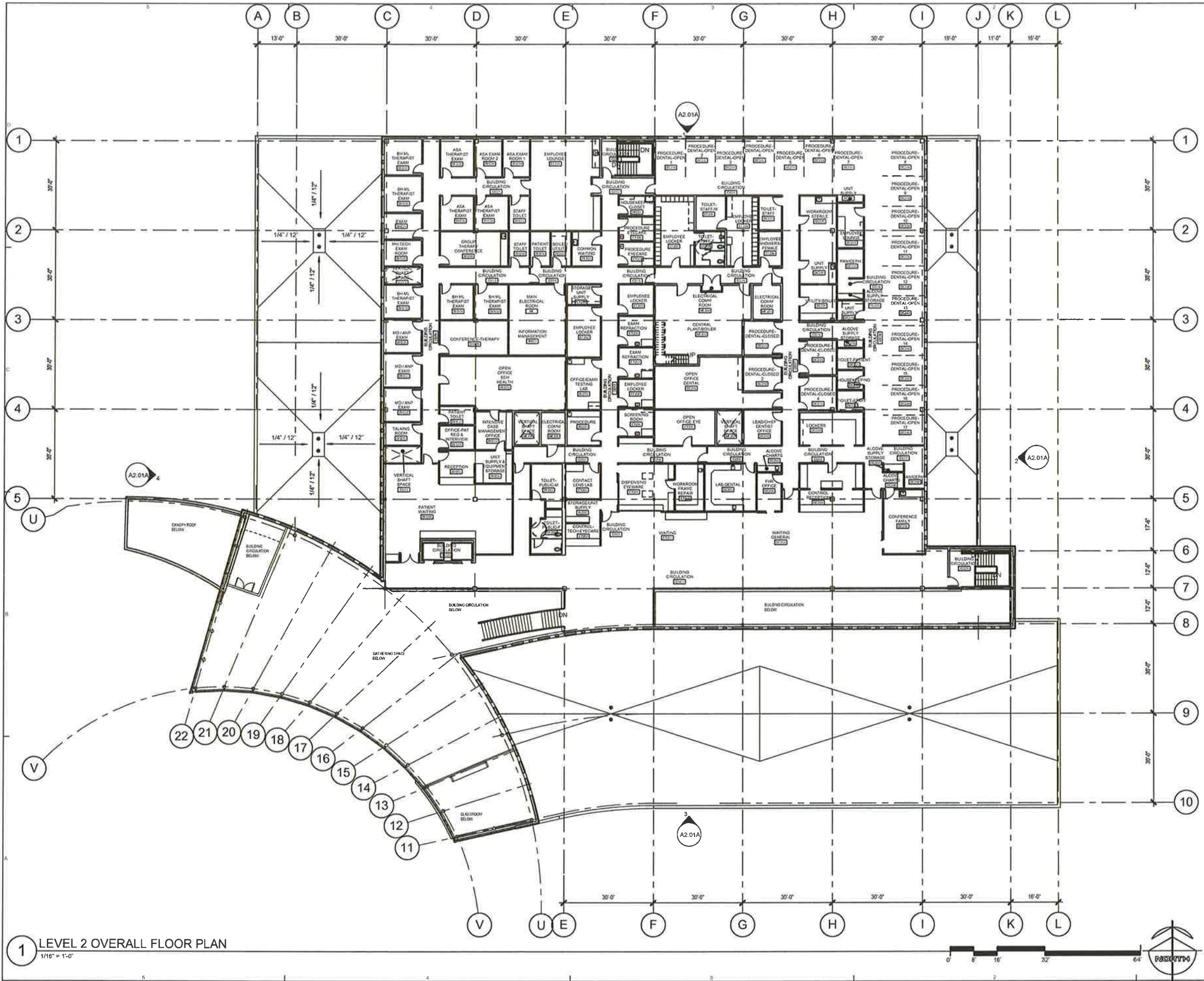
Southcentral Foundation
Valley Native Primary Care Center
 Wasilla, Alaska

REVISION	NO.	DESCRIPTION	DATE

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JOB NO. - kpb	A0061.01
JOB NO. - rbbj	100748.00
DATE	03/01/2011
DRAWN	Author
REVIEWED	Checker

SHEET NAME
 LEVEL 1 - OVERALL FLOOR
 PLAN
 SHEET NO.
A1.10

FULL SIZE: 37'-3 1/2" x 48'-0" HALF SIZE: 11'-7 1/2" x 17'-0"



1 LEVEL 2 OVERALL FLOOR PLAN
1/16" = 1'-0"



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kpb architects

nbbj

NEESER CONSTRUCTION, INC.

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Southcentral Foundation
Valley Native Primary Care Center
Wasilla, Alaska

REVISION	DESCRIPTION	DATE

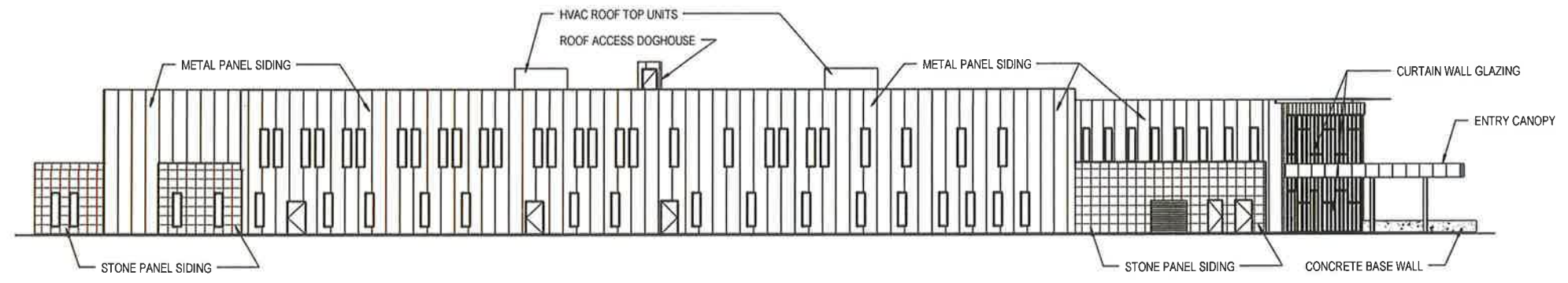
CONDITIONAL USE PERMIT APPLICATION

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JOB NO - kpb A9061 01
JOB NO - nbbj 100748 00
DATE 03/01/2011
DRAWN Author
REVIEWED Checker

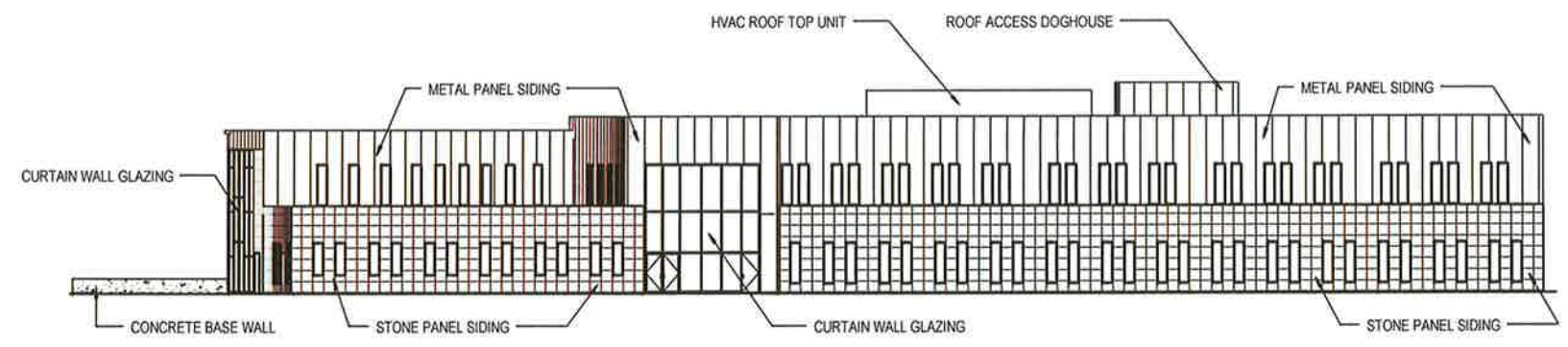
SHEET NAME
LEVEL 2 - OVERALL FLOOR PLAN

SHEET NO.
A1.20

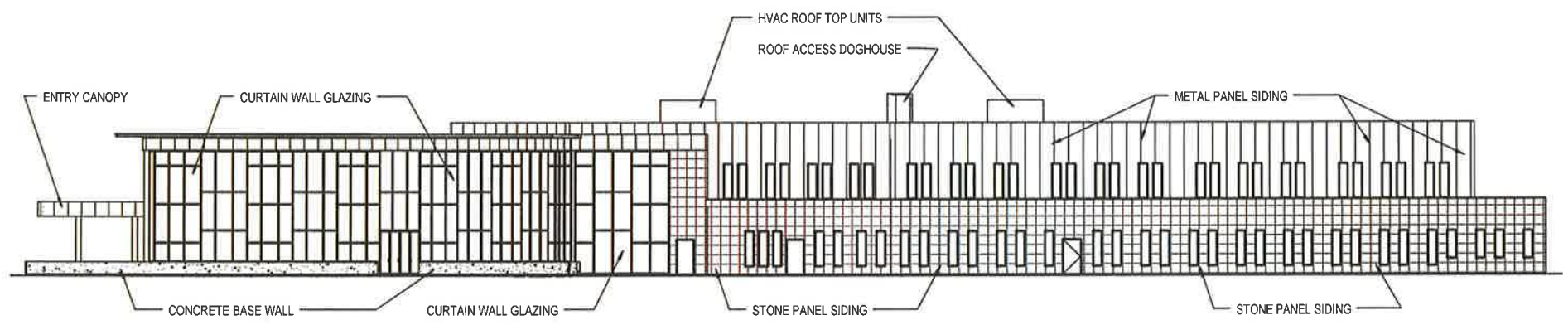
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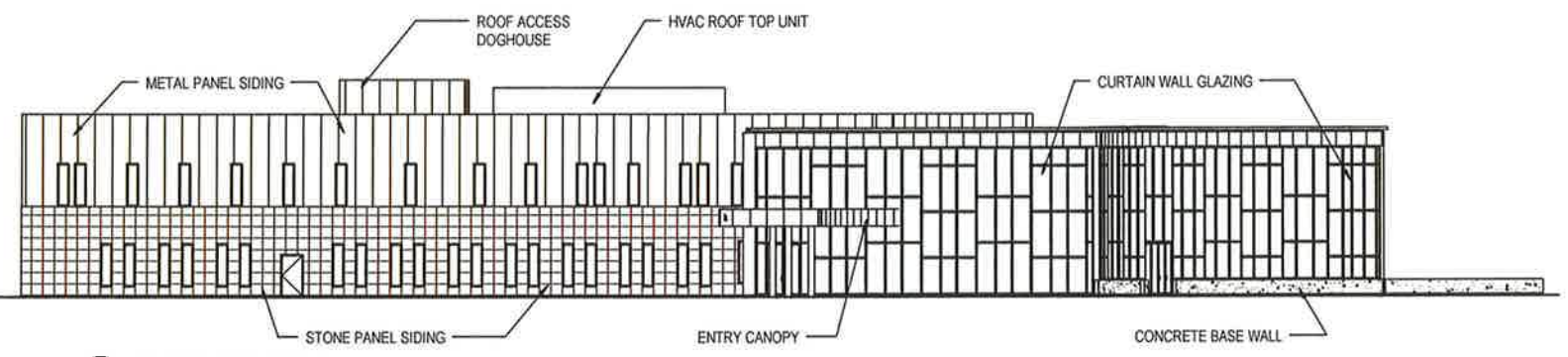
1 NORTH ELEVATION
1/16" = 1'-0"



2 EAST ELEVATION
1/16" = 1'-0"



3 SOUTH ELEVATION
1/16" = 1'-0"



4 WEST ELEVATION
1/16" = 1'-0"

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kp architects
mbbj
21500 1st Avenue North, Suite 200, Wasilla, Alaska 99567
Phone: (907) 274-8333 Fax: (907) 274-8333

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JOB NO. - rnbj	100746.00
DATE	03/01/2011
DRAWN	ghm
REVIEWED	Checker

SHEET NAME
EXTERIOR ELEVATIONS

SHEET NO.
A2.01A

CONDITIONAL USE PERMIT APPLICATION

FULL SIZE: 22" x 34" - HALF SIZE: 11" x 17"